

Round 3 Firth of Forth Development Zone

Pile Driving Analysis – Additional Assessment including Drive-Drill-Drive Mode

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SUMMARY

Seagreen have commissioned Cathie Associates to produce a pile driving assessment for the Marr Bank 1 and 2 provinces of the Phase 1 of Round 3 Firth of Forth Offshore Wind Development. The purpose of this assessment is to provide supporting information for the environmental impact assessment, including impact energies, blow counts and the likely pile driving equipment required.

1 INTRODUCTION

Cathie Associates have been commissioned by Seagreen to undertake a preliminary pile driving assessment for the Marr Bank 1 and Marr Bank 2 provinces of the Phase 1 Firth of Forth Offshore Wind Development. This assessment has been undertaken to provide noise data for input into the Environmental Impact Assessment (EIA).

A preliminary pile driving assessment for all areas of Phase 1 was produced and reported as a technical memo in September 2011, reference C162T24-01 ^[1]. This initial technical memo provided preliminary pile driving data for the entire Phase 1, based on the derivation of four ground models.

This additional technical memo reports the results of the assessment of pile driving completed for the provinces of Marr Bank 1 and 2 only. The locations of these provinces are shown in drawing reference C162R16-D01-01, Appendix A.

2 SCOPE OF WORKS

This assessment comprises the following work items:

1. Production of geotechnical profiles by:
 - The review of existing ground investigation reports and GIS database.
 - The use of two representative depth to bedrock geological scenarios:
 - Mean Case Bedrock (MCB) 12m below seabed.
 - Deep Case Bedrock (DCB) 20m below seabed.
 - The selection of geotechnical parameters (Upper Bound (UB) and Best Estimate (BE)) values for each geological scenario.
2. Use the pile design methodology recommended by DNV^[3] as implemented in the specialist software package 'OPILE' to determine the pile lengths required to support 6MW turbine loads for each scenario. 2m and 3m pile diameters and the following pile construction cases are to be considered:
 - Driving - Pile driving from seabed to target depth.
 - Drive Drill Drive - Pile driving from seabed to top of Triassic Group, drilling of rock socket and then driving to target depth through the rock socket.
3. Determine the preliminary Soil Resistance to Driving profiles (SRD) for each geotechnical profile and pile diameter (16 SRD's in total).
4. Conduct an assessment of pile driving using the specialist software package 'GRLWeap' and the derived SRD profiles.
5. Derive the following for each scenario, construction case, and pile diameter required:
 - Recommended hammer size.
 - Blow count per change in efficiency.
 - Installation time.

Work items 2 through 5 were also run considering 7MW turbine loads for the DCB UB scenarios for both pile construction cases.

3 DESIGN LOADS

The adopted design loads have been extracted from the Foundation Concept Engineering Study Report No: A4MRSEAG-Z-ENG945-SRP-084, supplied by Seagreen [2].

Design Case 11 was selected, as it models a typical 7MW turbine, with a jacket foundation in 50 m water depth. This is considered to be a conservative representation of the possible range of installation conditions potentially implemented on the Phase 1 area. Those loads were then reduced by 12% in order to obtain an estimate of the loadings corresponding to a 6MW turbine as instructed by Seagreen. The loads adopted for a 6MW turbine are as follows:

- Maximum ULS Pile Compression Load 34,070 kN
- Maximum ULS Pile Tension Load 25,970 kN

For a 7MW turbine the following loads were adopted:

- Maximum ULS Pile Compression Load 38,710 kN
- Maximum ULS Pile Tension Load 29,510 kN

These loads are understood to have been factored in accordance with DNV J101[3].

4 GEOLOGY OF MARR BANK ONE & MARR BANK TWO PROVINCES

The location and extent of Marr Bank 1 & 2 Provinces within Phase 1 of the Firth of Forth Offshore development is shown in drawing 162R16-D01-01, Appendix A. These provinces were defined by a maximum depth to rock head of 20m and the absence of Wee Bankie and Aberdeen Ground Formations.

4.1 Geological Units

The different geological units present in this site have been defined on the basis of a desk study and an intrusive ground investigation. The relevant information extracted for this assessment is summarised below.

4.1.1 Desk study and geophysical Survey

The initial desk study [4] indicated the geology of the entire Phase 1 area to comprise Holocene and Pleistocene deposits underlain by Triassic bedrock. The geophysical survey confirmed the presence of these formations and provided more detail regarding their extent and distribution.

The Holocene deposits encountered in the Marr Bank 1 and 2 provinces of Phase 1 comprise two formations: Undifferentiated Holocene sediments and Forth Formation. These formations are generally similar in composition predominantly comprising sand with occasional pockets of gravel. These two formations have been considered as Undifferentiated Holocene for the purposes of this assessment.

Marr Bank Formation is the only Pleistocene Formation present in Marr Bank 1 and 2 provinces. It is generally described as sand with abundant lithic gravel and pebbles. The lateral transition between Marr Bank formation (glacio-marine sediments) and Wee Bankie formation (glacial till) is situated to the west of Marr Bank provinces 1 & 2, and runs in a general north south orientation through the Phase 1 area. This transition is shown in drawing 162R16-D01-01.

The Triassic Group comprises sandstones, siltstones, mudstones and marls with thin sporadic bands of gypsum.

4.1.2 Findings of the Geotechnical Survey

The Phase 1 Geological Report is being prepared and has not as yet been issued. However the available borehole logs, CPT test data and laboratory testing completed to date has enabled a preliminary geological and geotechnical characterisation of the different formations for the purpose of this assessment. The formations encountered across the Marr Bank 1 & 2 Provinces can be typically described as follows:

- Undifferentiated Holocene. Loose to dense yellow and greyish brown silty fine to coarse SAND with shells, with occasional subordinate soft to stiff slightly silty clays.
- Marr Bank. Very dense dark grey silty fine SAND with sporadic pockets of clay, organic material and occasional silt layers.
- Undifferentiated Triassic. Extremely to moderately weak reddish brown laminated highly weathered SILTSTONE. Subordinate layers and units of very weak reddish brown fine grained SANDSTONE.

4.2 Geological Scenarios

The scenarios were developed following a review of the available geophysical data and the preliminary borehole logs and CPT data provided by the geotechnical survey. This assessment has been completed on the basis of two representative ground profiles or 'scenarios', defined as follows:

- Mean Case Bedrock (MCB). Models the mean depth to rock head, calculated across both; Marr Bank 1 and Marr Bank 2 provinces. This case is considered to represent the most likely pile lengths required.
- Deep Case Bedrock (DCB). Models the maximum depth to rock head for Marr Bank 1 and Marr Bank 2 provinces, defined in previous studies as 20m below seabed. This case is likely to require longer piles.

The scenarios represent the range of ground conditions present within Marr Bank 1 & 2. The formation thicknesses were derived from the available GIS data and are summarised in Table 1.

Table 1 – Geological Scenarios

Geological Unit	Formation levels (m below mud line)	
	Deep Case Bedrock	Mean Case Bedrock
Unit 1 Undifferentiated Holocene	0 – 5.6	0-5.6
Unit 2a Wee Bankie	Not present	Not present
Unit 2b Marr Bank	5.6- 20	5.6-12.2
Unit 3 Aberdeen Ground	Not present	Not present
Unit 4 Undifferentiated Triassic	20+	12.2+

5 GEOTECHNICAL PARAMETERS

Geotechnical parameters are required to determine the requisite pile lengths and for the derivation of soil resistance to driving (SRD) profiles. The derivation of geotechnical parameter values was made from available borehole descriptions, in-situ test data, laboratory test results and engineering judgement.

The Marr Bank Formation soils are typically cohesionless, however a significant cohesive silt portion has been observed locally. It was not deemed representative to model this detail at this level of assessment. Therefore, the Marr Bank has been considered only as a cohesionless unit.

The Triassic Group is generally described as extremely to moderately weak mudstone and siltstone, with the exception of the northeast of the site where it was described as sandstone. The mudstone and siltstone has potential for remoulding during driving, therefore in the absence of advanced testing this formation was modelled as a very hard cohesive material.

Pile capacity and driving resistance were calculated using the same relative values, i.e. both UB or both BE. The combination of parameters used for the geotechnical profiles are detailed in Tables 2 and 3.

Table 2 – Mean Case Bedrock Geotechnical Parameters

Unit (Major Fraction)	Unit Top Depth (mbsb)	Unit Bottom Depth (mbsb)	Thickness (m)	Effective Unit Weight (kN/m ³)	Plasticity		Upper Bound Undrained Shear Strength (kPa)		Axial Resistance Parameters			
					Liquid Limit (%)	Plastic Limit (%)	Top of Unit (kPa)	Bottom of Unit (kPa)	Max Skin Friction (kPa)	Max End Bearing (kPa)	Friction Angle (degrees)	Bearing Capacity Factor
Mean Case Bedrock - Best Estimate Parameters												
Undifferentiated Holocene (SAND)	0	5.6	5.6	11	-	-	-	-	96	9600	30	40
Marr Bank (SAND)	5.6	12.2	6.6	9	-	-	-	-	96	9600	30	40
Undifferentiated Triassic (Modelled as CLAY)	12.2	>50m	>50m	12	38	21	1000	1200	-	-	-	-
Mean Case Bedrock - Upper Bound Parameters												
Undifferentiated Holocene (SAND)	0	5.6	5.6	11	-	-	-	-	96	9600	30	40
Marr Bank (SAND)	5.6	12.2	6.6	10	-	-	-	-	115	12000	35	50
Undifferentiated Triassic (Modelled as CLAY)	12.2	>50m	>50m	13.5	38	21	1000	1500	-	-	-	-

Table 3 – Deep Case Bedrock Geotechnical Parameters

Unit (Major Fraction)	Unit Top Depth (mbsb)	Unit Bottom Depth (mbsb)	Thickness (m)	Effective Unit Weight (kN/m ³)	Plasticity		Upper Bound Undrained Shear Strength (kPa)		Axial Resistance Parameters			
					Liquid Limit (%)	Plastic Limit (%)	Top of Unit (kPa)	Bottom of Unit (kPa)	Max Skin Friction (kPa)	Max End Bearing (kPa)	Friction Angle (degrees)	Bearing Capacity Factor
Deep Case Bedrock- Best Estimate Parameters												
Undifferentiated Holocene (SAND)	0	5.6	5.6	11	-	-	-	-	96	9600	30	40
Marr Bank (SAND)	5.6	20	6.6	9	-	-	-	-	96	9600	30	40
Undifferentiated Triassic (Modelled as CLAY)	20	>50m	>50m	12	38	21	1000	1200	-	-	-	-
Deep Case Bedrock - Upper Bound Parameters												
Undifferentiated Holocene (SAND)	0	5.6	5.6	11	-	-	-	-	96	9600	30	40
Marr Bank (SAND)	5.6	20	6.6	10	-	-	-	-	115	12000	35	50
Undifferentiated Triassic (Modelled as CLAY)	20	>50m	>50m	13.5	38	21	1000	1500	-	-	-	-

6 PILE DESIGN

6.1 Methodology

The preliminary pile design followed the methodologies recommended by the standard DNV J101 [3] and was implemented with the Cathie Associates' commercial software package OPile [5]. Relevant details of this methodology are presented in the following sections.

6.1.1 Evaluation of Static Skin Friction & End Bearing Capacity

The ultimate bearing capacity of a single pile, Q_d , is determined from the following equation [3].

$$Q_d = Q_f + Q_p = \sum f A_s + q A_a$$

Where:

- Q_f = skin friction, kN,
- Q_p = total end bearing, kN,
- f = unit skin friction capacity, kPa,
- A_s = external pile shaft area, m²,
- q = unit end bearing capacity, kPa,
- A_a = base area of pile, m².

The base area is only taken into account if the pile is calculated as being plugged during installation and at the final target penetration. In the unplugged condition, the internal skin friction is taken into account to quantify the total shaft resistance.

6.1.2 Cohesive Soils

The (static) unit shaft friction in clay, f , is calculated by the following empirical equation for total stress methods [3]:

$$f = \alpha S_u$$

Where:

S_u is the undrained shear strength and α is the adhesion factor calculated by:

$$\alpha = 0.5 (S_u/p')^{-0.5} \quad \text{for } (S_u/p') \leq 1.0$$

$$\alpha = 0.5 (S_u/p')^{-0.25} \quad \text{for } (S_u/p') > 1.0$$

Where:

p' is the effective overburden pressure, in kPa, at the depth being considered.

For piles end bearing in cohesive soils, the unit end bearing, q , in kPa is computed from:

$$q = 9S_u$$

6.1.3 Cohesionless Soils

For pipe piles in cohesionless soils, the shaft friction, f , in kPa, is calculated as follows:

$$f = k p_o \tan \delta$$

Where:

-
- K = coefficient of lateral earth pressure at rest, taken as 0.8 for tension and compression,
- p_o = effective overburden pressure, kPa, at the depth being considered,
- δ = friction angle between the soil and the pile wall.

For piles end bearing in cohesionless soil, the unit end bearing, q may be computed from the following:

$$q = p_o N_q$$

Where:

N_q is a dimensionless bearing capacity factor.

Limiting values of skin friction and end bearing were applied in cohesionless soil as per DNV J101 recommendations.

A factor of safety of 1.25 on the characteristic soil resistances calculated was applied to derive the allowable soil resistance used to determine the final pile length.

Two construction methods were considered:

- Piles driven to depth. The smallest of the plugged and unplugged capacity calculations was selected as design resistance value.
- Piles driven to bedrock, drilled to final depth and driven through the rock socket. In this case the internal skin friction was reduced and the end bearing was calculated considering the annulus of the pile only.

6.2 Pile Sizing

Seagreen's conceptual design engineering was based on the following pile sizes:

- 2m diameter, 60mm wall thickness
- 3m diameter, 60mm wall thickness

The required pile lengths and diameters for the scenarios considered are summarised below and example calculations are contained in Appendix B.

Table 4 –Pile Lengths Adopted for Driving Assessment

Scenario (Geotechnical parameters)	2m Diameter Pile Length (m)	3m Diameter Pile Length (m)
Pile Drive Mode		
MCB BE 6MW	27	22
MCB UB 6MW	25	21
DCB BE 6MW	32	27
DCB UB 6MW	30	26
DCB UB 7MW	32	27
Drive-Drill-Drive Mode		
MCB BE 6MW	29	24
MCB UB 6MW	27	22
DCB BE 6MW	35	29
DCB UB 6MW	32	27
DCB UB 7MW	34	29

The 2m diameter piles require a greater length to support the loads applied. The pile lengths calculated for the drive-drill-drive mode are slightly longer as would be expected from the minimal internal skin friction and end bearing area limited to the pile annulus.

7 PILE DRIVING ASSESSMENT

A pile driving assessment was undertaken with the wave equation analysis method on the basis of estimated soil resistance to driving profiles (SRD) for each scenario, set of geotechnical properties, loadings and pile size. The wave equation method models the impact energy and blow counts required to drive the piles to the required depths. This model was completed using the software program GRLWEAP [6].

7.1 Soil Resistance to Driving

The soil resistance to driving (SRD) profiles are estimated from empirical relationships with static resistance as explained in the following sections.

7.1.1 Sand (Undifferentiated Holocene and Marr Bank Formation)

The static shaft friction and end bearing capacities are calculated following the DNV method [3]. The limiting skin frictions and end bearing values specified in DNV J101 for granular materials were also adopted. However, for sand, the dynamic resistance factor on static shaft friction is taken as 0.7 in accordance to the Stevens Method [7].

$$\begin{aligned}\text{Dynamic Shaft Capacity, SRD} &= 0.7 \times \text{Static Capacity} \\ \text{Dynamic End bearing Capacity, SRD} &= \text{End Bearing Capacity}\end{aligned}$$

7.1.2 Clay (Triassic Group)

The empirical method proposed by Stevens [7] was adopted to estimate the shaft resistance of this Group during driving. The static shaft resistance was estimated from DNV J101 [3] and then reduced by applying a Dynamic Resistance Factor (F_p) to calculate the dynamic shaft resistance.

$$F_p = 0.5(\text{OCR})^{0.3}$$

Where,

$$\text{OCR} = \frac{\text{Over-consolidation ratio}}{[S_{u(oc)} / S_{u(nc)}]^{1/0.85}}$$

$$S_{u(oc)} = \text{Undrained shear strength of over-consolidated clay,}$$

$$S_{u(nc)} = \text{Undrained shear strength of normally-consolidated clay, where:}$$

In calculating OCR, the value of $S_{u(nc)}$ is calculated using the effective overburden stress, p'_0 , from the following equation:

$$S_{u(nc)} = \sigma_v (0.11 + 0.0037PI), \text{ and}$$

$$PI = \text{Plasticity Index}$$

Mercia Mudstone behaves as a 'lightly' to 'moderately' over consolidated clay[8]. An OCR of 1 has been adopted for this formation in the absence of detailed geotechnical testing.

In summary the following Dynamic Resistance Factors were applied for the Triassic Group:

$$\text{Shaft capacity, SRD} = F_p \times \text{Static Capacity}$$

End bearing Capacity, SRD = 1.0 x End Bearing Capacity

7.1.3 Influence of construction methods

Driven piles

The pile driving mode is typically unplugged (coring) when piles are initially driven. On further penetration, the pile driving mode may become 'plugged' when the internal shaft resistance (inside the pile) exceeds the end bearing resistance acting on the area of soil within the pile annulus. It should be noted that plugged piles may subsequently unplug with further driving. This typically occurs when significant increases in end bearing resistance are encountered. The depth of plugging cannot be predicted accurately and depends on local soil conditions, driving energy and pile diameter.

The final SRD profile was based on the smaller soil capacity obtained when comparing the plugged and unplugged modes in their static condition.

Drive-drill-drive

As detailed in Section 2, the pile shall be driven through Holocene and Pleistocene sediment to bedrock, a rock socket will then be drilled and the pile driven to target depth through the rock socket. Therefore, the internal shaft resistance to driving was ignored when deriving the SRD for piles drilled through the Triassic Group. Drilled Sockets of 1m and 1.25m diameter were considered for 2m diameter and 3m diameter piles respectively in order to quantify the volume of debris accumulated inside the hollowed pile when driving through a drilled socket. It was assumed that the debris would generate a magnitude of internal skin friction proportional to its own volume.

7.1.4 Influence of soil set up

The effects of increasing axial pile capacity with time and enhanced resistance to driving are documented in numerous papers [9 & 10]. The construction of drilled sockets in the Triassic Group would require the use of drilling techniques. The change of construction techniques from driving to drilling and finally driving through the rock socket may take several hours or, in case of unfavourable weather conditions, several days.

A set up factor of 2 was applied to the total external skin friction and end bearing values in the first 2m of the Triassic Group to calculate the SRD in the case of drive-drill-drive mode. This assumption is based on the data presented by Jardine et al (2005), 'ICP methods for driven piles in sands and clays' [9] and the approach discussed in Skov R and Denver H (1988), 'Time-dependence of bearing capacity of piles Proc. 3rd Int. Conf. on Application of Stress-wave Theory to Piles (Ottawa, Canada, 25-27 May 1988)' [10] and is considered adequate for delays up to 20 days between drilling and driving.

7.2 Pile Driving Assessment

The SRD profiles, pile sizes and target depths were modelled in GRLWEAP for a range of hammer sizes.

A maximum hammer operating efficiency of 95% was used to define the likely hammer size required to drive the piles. A hammer efficiency of 95% corresponds to global efficiencies of approximately 90%. The global efficiency is defined as the ratio between the ENTHRU energy (energy transmitted to the pile) and the maximum rated energy of the hammer.

The wave equation analysis shall use the following input parameters, adopted from the preliminary pile driving assessment for all areas of Phase 1, reference C162T24-01 [1]:

Table 5 – Wave Equation Parameters

Parameter	Cohesive	Cohesionless
Quake – shaft	2.5mm	2.5mm
Quake – toe	2.5mm	2.5mm
Damping – shaft	0.2	0.2
Damping – toe	0.5	0.5

A preliminary pile driving assessment was completed by assuming a rate of 400 blows per metre as the refusal criteria for each scenario to determine the hammer size. This preliminary assessment of hammer size assumed the piling hammer to be operating at 95% efficiency for the entire duration of the installation operations.

A more detailed driving assessment was then completed for each scenario and pile size as follows:

- An initial hammer efficiency of 15% is applied from mud line level until a limit of 100 blows per meter is achieved.
- Once the limit is achieved, the hammer efficiency is increased in 20% intervals to final penetration.

The results of the pile drivability assessment were exported to a table, which is presented in Appendix C. A summary of the results is presented in tables 6 & 7 below.

Table 6 – Pile driving to full depth.

Scenario	Pile diameter (m)	Piling Summary						
		Penetration Depth (m)	Max SRD (MN)	Hammer	Hammer efficiency	Max Impact Energy (kJ)	Total No. of blows	Duration (hours)
MCB BE 6MW	2	27	78.5*	IHC-S1800	15-95%	1420	2316	0.9
	3	22	61	IHC-S1800	15-95%	1436	1329	0.5
MCB UB 6MW	2	25	90.8**	IHC S2300	15-95%	2056	1553	0.6
	3	21	67.6	IHC-S2300	15-95%	2081	893	0.3
DCB BE 6MW	2	32	78.1***	IHC-S1800	15-95%	1432	2381	0.9
	3	27	56.6	IHC-S1800	15-95%	1449	1406	0.5
DCB UB 6MW	2	30	58.3	IHC-S1800	15-95%	1445	1597	0.6
	3	26	63.2	IHC-S1800	15-95%	1446	1393	0.5
DCB UB 7MW	2	<32	Refusal with maximum hammer size of IHC S2300****					
	3	27	69	IHC 1800	15-95%	1449	1449	0.5
Range	2-3	21-32	56.6-91	IHC 1800	15-95%	1420-2081	893-2381	0.5-0.9

*Plugged at 24m, **Plugged at 25m, ***Plugged at 30m, ****Maximum hammer considered, assumes plugging at depth.

Table 7 – Pile driving to bedrock and drilled socket in the Triassic Group

Scenario	Pile diameter (m)	Piling Summary						
		Penetration Depth (m)	Max SRD (MN)	Hammer	Hammer efficiency	Max Impact Energy (kJ)	Total No. of blows	Duration (hours)
MCB BE 6MW	2	29	40.8	IHC-S1200	15-75%	916	1445	0.5
	3	24	40.3	IHC-S1200	15-75%	920	1362	0.5
MCB UB 6MW	2	27	40.8	IHC-S1200	15-75%	915	1351	0.5
	3	22	49	IHC-S1200	15-75%	919	1081	0.4
DCB BE 6MW	2	35	39.4	IHC-S1200	15-75%	870	2061	0.8
	3	29	46.3	IHC-S1200	15-75%	870	1746	0.6
DCB UB 6MW	2	32	40.3	IHC-S1200	15-75%	869	1802	0.7
	3	27	47.7	IHC-S1200	15-75%	869	1662	0.6
DCB UB 7MW	2	34	45.6	IHC-S1200	15-95%	1100	2062	0.8
	3	29	55.6	IHC-S1200	15-95%	1099	2199	0.8
Range	2-3	22-34	39.4-55.6	IHC-S1200	15-95%	870-1100	1081-2199	0.5-0.9

The piling durations have been preliminarily assessed by assuming a hammer frequency of 45 blows/meter and uninterrupted driving operations. The total durations anticipated (assuming continuous driving) are presented in Appendix C. A summary of the results is presented in Table 7.

The Triassic Group shows significantly higher resistance to driving than the overlying Marr Bank Formation and Undifferentiated Holocene Formation.

8 INDICATIVE PILING DURATIONS

Indicative piling operation timings for drive-drill-drive and driven piling are provided in Table 8. These timings have been derived from records from recent piling operations in similar ground conditions (source confidential); however piling operations vary significantly depending upon the contractor, installation vessel, weather and plant. The following assumptions have been made in the derivation of these timings:

- The installation vessel is a self-propelled jack-up.
- A single four pile jacket structure is the sub-structure
- The drilling operation is top driven.
- The drill bit diameter is 1.7m.
- The drilling operation is a single run.
- No significant problems and optimum weather conditions.
- Deep bedrock case, 2m diameter pile, with a toe depth of 27m BSBL.

Table 8 – Indicative Pile Timings

Operation	Driven Piling Operations				Drive Drill Drive Piling Operations			
	Pile 1	Pile 2	Pile 3	Pile 4	Pile 1	Pile 2	Pile 3	Pile 4
On-site and Jack-up	2hrs	2hrs	2hrs	2hrs	2hrs	2hrs	2hrs	2hrs
Deployment and set-up of piling template	4hrs	NA	NA	NA	4hrs	NA	NA	NA
Pile placement and self-weight settle	2hrs	2hrs	2hrs	2hrs	2hr	2hrs	2hrs	2hrs
Piling set-up and setting blows	2hrs	2hrs	2hrs	2hrs	1.5hrs	1.5hrs	1.5hrs	1.5hrs
Pile Driving	1hr	1hr	1hr	1hr	0.5hrs	0.5hrs	0.5hrs	0.5hrs
Remove Hammer and Install Drill Rig	NA	NA	NA	NA	6hrs	6hrs	6hrs	6hrs
Top Drill and Reaming	NA	NA	NA	NA	1.5hrs	1.5hrs	1.5hrs	1.5hrs
Remove drill rig and reinstall Hammer	NA	NA	NA	NA	4hrs	4hrs	4hrs	4hrs
Piling (re) set-up	NA	NA	NA	NA	1hr	1hr	1hr	1hr
Pile Driving (Restart)	NA	NA	NA	NA	1hr	1hr	1hr	1hr
Post Piling ops	1hr	1hr	1hr	1hr	1hr	1hr	1hr	1hr
Recovery of piling template	NA	NA	NA	3hrs	NA	NA	NA	3hr
Jack down	2hrs	2hrs	2hrs	2hrs	2hrs	2hrs	2hrs	2hrs
Transfiting to next location and positioning	1hr	1hr	1hr	NA/Unknown	1hr	1hr	1hr	NA/Unknown
Per pile Total	15hrs	11hrs	11hrs	13hrs	41hrs	37hrs	37hrs	39hrs
JACKET TOTAL	50hrs				154hrs			

9 CONCLUSIONS AND RECOMMENDATIONS

The following conclusions can be drawn from this assessment:

- A significant range of driving energies, blow counts and durations can be expected during the piling work operations.
- The range of impact energies associated with drive-drill-drive mode is significantly lower than on driven mode only.
- The MCB UB 6MW scenario for full depth pile driving requires very high impact energies to achieve target penetration. Due to a number of issues, including the noise generated and its potential effect on marine mammals, this installation scenario is not considered to be a feasible method of installing piles in these circumstances. Therefore, for this scenario drive-drill-drive installation should be considered.
- The delays associated with changes between driving and drilling techniques cannot be predicted accurately and will depend on the contractors' method of work, however significant 'pile set-up' is expected.
- The possibility of plugging, particularly on the 2m diameter piles when driven through the Triassic Group cannot be accurately predicted.
- The use of 2m diameter piles will result in a risk of refusal before target depth and adequate pile capacity is achieved.

This is a preliminary assessment based upon preliminary ground investigation information. We recommend that this assessment is re-visited on receipt of more detailed geological and/or geotechnical data.

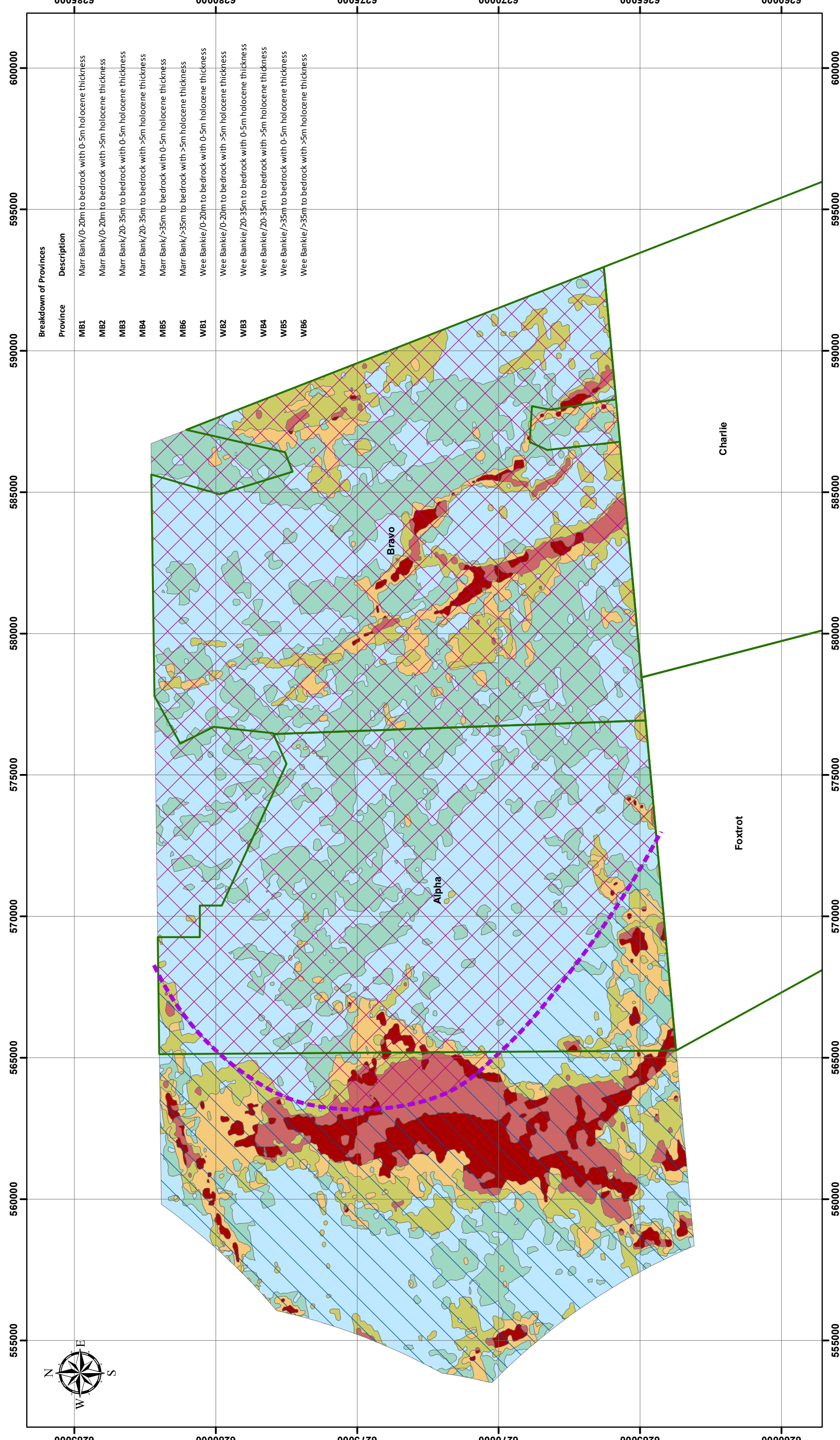
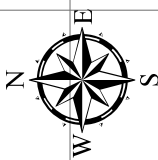
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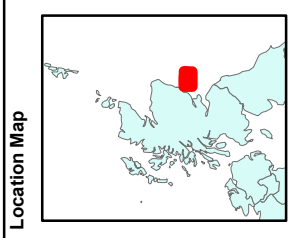
Appendix A

Location of Marr Bank 1 & Marr Bank 2 Provinces.

Drawing 162R16-001-01



Breakdown of Provinces	
Province	Description
MB1	Marr Bank/0-20m to bedrock with 0-5m holocene thickness
MB2	Marr Bank/0-20m to bedrock with >5m holocene thickness
MB3	Marr Bank/20-35m to bedrock with 0-5m holocene thickness
MB4	Marr Bank/20-35m to bedrock with >5m holocene thickness
MB5	Marr Bank/>35m to bedrock with 0-5m holocene thickness
MB6	Marr Bank/>35m to bedrock with >5m holocene thickness
WB1	Wee Bankie/0-20m to bedrock with 0-5m holocene thickness
WB2	Wee Bankie/0-20m to bedrock with >5m holocene thickness
WB3	Wee Bankie/20-35m to bedrock with 0-5m holocene thickness
WB4	Wee Bankie/20-35m to bedrock with >5m holocene thickness
WB5	Wee Bankie/>35m to bedrock with 0-5m holocene thickness
WB6	Wee Bankie/>35m to bedrock with >5m holocene thickness



Legend

- Split Between Marr Bank and Wee Bankie

Province

MB1
MB2
MB3
MB4
MB5
MB6
WB1
WB2
WB3
WB4
WB5
WB6

Projection/Scale
 WGS_1984_UTM_Zone_30N
 Projection: Transverse_Mercator
 False_Easting: 500000.000000
 False_Northing: 0.000000
 Central_Meridian: -3.000000
 Scale_Factor: 0.999800
 Latitude_Of_Origin: 0.000000
 Linear_Unit: Meter
 Datum: D_WGS_1984

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CATHIE ASSOCIATES

Revision Status		Details of Changes		Drn Chk
Rev	Date	First Issue	PSL MRC	
1	15/02/12			

Sea green WIND ENERGY

Firth of Forth Offshore Wind Farm

R16 - Phase 1 Development Zone Interpretive Report

Province Map

C162R16-D01-01 | PSL | Rev 1

Appendix B

Example of Pile Sizing Calculations

PILE DRIVING ASSESSMENT
 Pile Sizing Calculations
 DMCB_BE 6MW



Pile Diameter (m)	Jack-Hit	
	Compress (kN)	Tension (kN)
2m	34.07	25.97
3m	34.07	25.97

Pile Diameter (m)	Capacity			Utilisation		Hammer Type/Size
	Compress	Tens	Total	Compress	Tens	
2m	51.80	27.50	0.66	0.94	HIC-S1800	
3m	55.97	26.52	0.61	0.98	HIC-S1800	

1.25 resistance factor applied to soil resistances (DNV-OS-1001)

2m Pile Diameter

Final Pen (m)	End Capacity		Cum Friction		Factored Tension		Total Capacity		Utilisation		Hammer Type/Size
	Plugged (kN)	Unplugged (kN)	Compress (kN)	Tension (kN)	Plugged (kN)	Unplugged (kN)	Plugged (kN)	Unplugged (kN)	Compress	Tens	
0	0	0	0	0	0	0	0	0	0	0	HIC-S1800
1	0	0	0	0	0	0	0	0	0	0	HIC-S1800
2	0	0	0	0	0	0	0	0	0	0	HIC-S1800
3	4147	516	36	36	28.6	52	4182.7	552	UNPLUGGED	UNPLUGGED	HIC-S1800
4	5529	782	147	147	117.984	930	5676.2	744	UNPLUGGED	UNPLUGGED	HIC-S1800
5	6912	1078	291	291	232.912	1369	7206.1	1095	UNPLUGGED	UNPLUGGED	HIC-S1800
6	7741	1270	393	393	314.12	1835	8133.5	1463	UNPLUGGED	UNPLUGGED	HIC-S1800
7	8872	1582	584	584	467.584	2453	9456.3	2167	UNPLUGGED	UNPLUGGED	HIC-S1800
8	9324	1760	669	669	534.816	2982	9992.8	2666	UNPLUGGED	UNPLUGGED	HIC-S1800
9	10455	2040	897	897	717.536	3957	11352.1	3707	UNPLUGGED	UNPLUGGED	HIC-S1800
10	12217	2824	1432	1432	1148.68	5382	13737.6	5096	UNPLUGGED	UNPLUGGED	HIC-S1800
11	13848	3264	1739	1739	1391.04	6229	15597.3	5888	UNPLUGGED	UNPLUGGED	HIC-S1800
12	15205	3783	2041	2041	1715.12	6924	17044.5	6708	UNPLUGGED	UNPLUGGED	HIC-S1800
13	16424	4288	2341	2341	2116.32	7483	18188.8	7566	UNPLUGGED	UNPLUGGED	HIC-S1800
14	17514	4784	2638	2638	2593.6	7915	19074.4	8461	UNPLUGGED	UNPLUGGED	HIC-S1800
15	18483	5271	2933	2933	3157.0	8218	19725.0	9391	UNPLUGGED	UNPLUGGED	HIC-S1800
16	19341	5749	3227	3227	3807.6	8400	20160.6	10364	UNPLUGGED	UNPLUGGED	HIC-S1800
17	20097	6220	3521	3521	4555.2	8482	20405.8	11489	UNPLUGGED	UNPLUGGED	HIC-S1800
18	20763	6684	3816	3816	5409.8	8454	20474.6	12764	UNPLUGGED	UNPLUGGED	HIC-S1800
19	21348	7144	4112	4112	6371.4	8318	20380.0	14300	UNPLUGGED	UNPLUGGED	HIC-S1800
20	21863	7600	4409	4409	7440.0	8074	20132.0	16020	UNPLUGGED	UNPLUGGED	HIC-S1800
21	22317	8056	4707	4707	8616.6	7724	19750.0	17960	UNPLUGGED	UNPLUGGED	HIC-S1800
22	22731	8512	5007	5007	9901.2	7280	19244.0	20140	UNPLUGGED	UNPLUGGED	HIC-S1800
23	23115	8976	5309	5309	11303.8	6732	18624.0	22600	UNPLUGGED	UNPLUGGED	HIC-S1800
24	23479	9448	5614	5614	12824.4	6190	17900.0	25380	UNPLUGGED	UNPLUGGED	HIC-S1800
25	23824	9927	5923	5923	14473.0	5654	17084.0	28440	UNPLUGGED	UNPLUGGED	HIC-S1800
26	24150	10415	6235	6235	16250.6	5124	16188.0	31840	UNPLUGGED	UNPLUGGED	HIC-S1800
27	24469	10912	6551	6551	18167.2	4609	15224.0	35640	UNPLUGGED	UNPLUGGED	HIC-S1800
28	24783	11428	6881	6881	20223.8	4109	14216.0	40000	UNPLUGGED	UNPLUGGED	HIC-S1800
29	25093	11963	7225	7225	22431.4	3624	13172.0	45000	UNPLUGGED	UNPLUGGED	HIC-S1800
30	25400	12516	7583	7583	24791.0	3154	12100.0	50800	UNPLUGGED	UNPLUGGED	HIC-S1800
31	25705	13088	7956	7956	27312.6	2708	11016.0	57400	UNPLUGGED	UNPLUGGED	HIC-S1800
32	26009	13679	8344	8344	29995.2	2287	9928.0	64800	UNPLUGGED	UNPLUGGED	HIC-S1800
33	26312	14288	8748	8748	32838.8	1891	8864.0	74000	UNPLUGGED	UNPLUGGED	HIC-S1800
34	26615	14915	9168	9168	35843.4	1520	7912.0	84000	UNPLUGGED	UNPLUGGED	HIC-S1800
35	26918	15560	9604	9604	39009.0	1174	7072.0	95000	UNPLUGGED	UNPLUGGED	HIC-S1800
36	27221	16223	10056	10056	42335.6	853	6344.0	107000	UNPLUGGED	UNPLUGGED	HIC-S1800
37	27525	16905	10534	10534	45823.2	557	5728.0	120000	UNPLUGGED	UNPLUGGED	HIC-S1800
38	27830	17607	11038	11038	49471.8	286	5232.0	134000	UNPLUGGED	UNPLUGGED	HIC-S1800
39	28135	18338	11568	11568	53281.4	49	4856.0	150000	UNPLUGGED	UNPLUGGED	HIC-S1800
40	28440	19099	12124	12124	57352.0	111	4500.0	168000	UNPLUGGED	UNPLUGGED	HIC-S1800
41	28745	19889	12706	12706	61683.6	18	4164.0	188000	UNPLUGGED	UNPLUGGED	HIC-S1800
42	29050	20708	13314	13314	66285.2	28	3848.0	210000	UNPLUGGED	UNPLUGGED	HIC-S1800
43	29355	21557	13948	13948	71156.8	41	3552.0	234000	UNPLUGGED	UNPLUGGED	HIC-S1800
44	29660	22436	14608	14608	76298.4	57	3276.0	260000	UNPLUGGED	UNPLUGGED	HIC-S1800
45	29965	23345	15294	15294	81700.0	77	3020.0	288000	UNPLUGGED	UNPLUGGED	HIC-S1800
46	30270	24284	16006	16006	87361.6	102	2784.0	318000	UNPLUGGED	UNPLUGGED	HIC-S1800
47	30575	25253	16744	16744	93283.2	133	2568.0	350000	UNPLUGGED	UNPLUGGED	HIC-S1800
48	30880	26252	17518	17518	99464.8	171	2372.0	384000	UNPLUGGED	UNPLUGGED	HIC-S1800
49	31185	27281	18328	18328	105906.4	217	2196.0	420000	UNPLUGGED	UNPLUGGED	HIC-S1800
50	31490	28340	19174	19174	112618.0	272	2040.0	458000	UNPLUGGED	UNPLUGGED	HIC-S1800

3m Pile Diameter

Final Pen (m)	End Capacity		Cum Friction		Factored Tension		Total Capacity		Utilisation		Hammer Type/Size
	Plugged (kN)	Unplugged (kN)	Compress (kN)	Tension (kN)	Plugged (kN)	Unplugged (kN)	Plugged (kN)	Unplugged (kN)	Compress	Tens	
0	0	0	0	0	0	0	0	0	0	0	HIC-S1800
1	0	0	0	0	0	0	0	0	0	0	HIC-S1800
2	0	0	0	0	0	0	0	0	0	0	HIC-S1800
3	0	0	0	0	0	0	0	0	0	0	HIC-S1800
4	1240.7	93.51	18.91	18.91	93.51	42	1240.7	52	UNPLUGGED	UNPLUGGED	HIC-S1800
5	1550.9	144.2	28.39	28.39	144.2	63	1550.9	77	UNPLUGGED	UNPLUGGED	HIC-S1800
6	1741	213.2	38.67	38.67	213.2	90	1741	105	UNPLUGGED	UNPLUGGED	HIC-S1800
7	2027.2	226.9	46.76	46.76	269.7	123	2027.2	139	UNPLUGGED	UNPLUGGED	HIC-S1800
8	2488.1	307.1	61.81	61.81	371.9	167	2488.1	186	UNPLUGGED	UNPLUGGED	HIC-S1800
9	2911.9	438.2	83.72	83.72	500.1	222	2911.9	247	UNPLUGGED	UNPLUGGED	HIC-S1800
10	3101.8	583.2	111.3	111.3	651.4	297	3101.8	326	UNPLUGGED	UNPLUGGED	HIC-S1800
11	3252.8	793.2	150.4	150.4	844.6	394	3252.8	434	UNPLUGGED	UNPLUGGED	HIC-S1800
12	3392.1	1048.2	201.4	201.4	1076.0	524	3392.1	580	UNPLUGGED	UNPLUGGED	HIC-S1800
13	3517.2	1380.6	266.4	266.4	1362.4	697	3517.2	787	UNPLUGGED	UNPLUGGED	HIC-S1800
14	3631.2	1819.6	350.4	350.4	1712.8	914	3631.2	1051	UNPLUGGED	UNPLUGGED	HIC-S1800
15	3734.2	2385.6	463.4	463.4	2176.2	1175	3734.2	1386	UNPLUGGED	UNPLUGGED	HIC-S1800
16	3826.2	3099.6	616.4	616.4	2762.6	1491	3826.2	1802	UNPLUGGED	UNPLUGGED	HIC-S1800
17	3907.2	3973.6	820.4	820.4	3483.0	1862	3907.2	2349	UNPLUGGED	UNPLUGGED	HIC-S1800
18	4000.2	5017.6	1084.4	1084.4	4347.4	2396	4000.2	3145	UNPLUGGED	UNPLUGGED	HIC-S1800
19	4106.2	6341.6	1409.4	1409.4	5456.8	3192	4106.2	4239	UNPLUGGED	UNPLUGGED	HIC-S1800
20	4234.2	8065.6	1805.4	1805.4	6911.2	4266	4234.2	5706	UNPLUGGED	UNPLUGGED	HIC-S1800
21	4384.2	10199.6	2381.4	2381.4	8892.6	5722	4384.2	7662	UNPLUGGED	UNPLUGGED	HIC-S1800
22	4556.2	12753.6	3147.4	3147.4	11490.0	7566	4556.2	10018	UNPLUGGED	UNPLUGGED	HIC-S1800
23	4750.2	16797.6	4114.4	4114.4	14824.4	10000	4750.2	13264	UNPLUGGED	UNPLUGGED	HIC-S1800
24	4976.2	22441.6	5394.4	5394.4	19118.8	13234	4976.2	17400	UNPLUGGED	UNPLUGGED	HIC-S1800
25	5244.2	29985.6	7118.4	7118.4	25237.2	17468	5244.2	22646	UNPLUGGED	UNPLUGGED	HIC-S1800
26	5566.2	40239.6	9422.4	9422.4	33261.6	23002	5566.2	30092	UNPLUGGED	UNPLUGGED	HIC-S1800
27	5954.2	54083.6	12496.4	12496.4	43306.0	30206	5954.2	39898	UNPLUGGED	UNPLUGGED	HIC-S1800
28	6410.2	72427.6	16540.4	16540.4	56846.4	39410	6410.2	52344	UNPLUGGED	UNPLUGGED	HIC-S1800
29	6946.2	97371.6	21904.4	21904.4	75150.8	51114	6946.2	69150	UNPLUGGED	UNPLUGGED	HIC-S1800
30	7574.2	130815.6	29138.4	29138.4	99289.2	66428	7574.2	91378	UNPLUGGED	UNPLUGGED	HIC-S1800
31	8298.2	174459.6	38692.4	38692.4	131223.6	86852	8298.2	119830	UNPLUGGED	UNPLUGGED	HIC-S1800
32	9122.2	230303.6	51326.4	51326.4	174550.0	113506	9122.2	156882	UNPLUGGED	UNPLUGGED	HIC-S1800
33	10060.2	301647.6	67470.4	67470.4	232020.4	148480	10060.2	207422	UNPLUGGED	UNPLUGGED	HIC-S1800
34	11238.2	390791.6	89464.4	89464.4	311484.8	194384	11238.2	275146	UNPLUGGED	UNPLUGGED	HIC-S1800
35	12682.2	504735.6	119408.4	119408.4	401993.2	254808	12682.2	362270	UNPLUGGED	UNPLUGGED	HIC-S1800
36	14426.2	662679.6	158352.4	158352.4	521345.6	333132	14426.2	473134	UNPLUGGED	UNPLUGGED	HIC-S1800
37	16506.2	874623.6	208796.4	208796.4	680642.0	434566	16506.2	622000	UNPLUGGED	UNPLUGGED	HIC-S1800
38	18986.2	1155067.6	276340.4	276340.4	896982.4	574000	18986.2	820000	UNPLUGGED	UNPLUGGED	HIC-S1800
39	21910.2	1516511.6	367384.4	367384.4	1184366.8	754434	21910.2	1070000	UNPLUGGED	UNPLUGGED	HIC-S1800
40	25334.2	1978955.6	487428.4	487428.4	1581795.2	995868	25334.2	1390000	UNPLUGGED	UNPLUGGED	HIC-S1800
41	29418.2	2603399.6	644472.4	644472.4							

PILE DRIVING ASSESSMENT
 Pile Sizing Calculations
 DCB BE - 7MW Loads



Pile Diameter	Jack-het Tension (MN)	
	Compress	Tension
2m	38.71	29.51
3m	38.71	29.51

Pile Diameter	Pile Length	Capacity		Hammer Type/Size
		Compress	Tens	
2m	31.93	0.63	0.92	
3m	61.79	0.60	0.97	

Pile Diameter	Pile Length	Utilisation	
		Compress	Tens
2m	31.93	0.63	0.92
3m	61.79	0.60	0.97

1.25 resistance factor applied to soil resistances (DNV-O5-100)

2m Pile Diameter

3m Pile Diameter

Final Pen (m)	End Capacity		Cum Friction	Total Capacity		Factored Total Capacity (kN)	Utilisation	Hammer Type/Size
	Plugged (kN)	Unplugged (kN)		Plugged (kN)	Unplugged (kN)			
0	0	0	0	0	0	0		
1	0	0	0	0	0	0		
2	0	0	0	0	0	0		
3	41.7	51.6	3.6	28.6	418.7	552	UNPLUGGED	
4	55.29	782	1.47	117.984	5675.6	930	UNPLUGGED	
5	69.12	1078	2.91	232.912	7202.6	1369	UNPLUGGED	
6	77.41	1270	3.93	314.12	8133.5	1643	UNPLUGGED	
7	89.98	1637	6.27	501.648	9624.6	2244	UNPLUGGED	
8	9500	1793	7.31	584.544	10230.9	2019	UNPLUGGED	
9	10757	2206	10.14	811.52	11771.2	3220	UNPLUGGED	
10	12014	2652	13.33	1065.56	13346.7	3985	UNPLUGGED	
11	14527	3443	20.77	1661.28	16603.3	5723	UNPLUGGED	
12	15783	4188	25.01	2000.8	18284.4	6489	UNPLUGGED	
13	17040	4767	29.41	2368.56	20007.7	7277	UNPLUGGED	
14	18297	5378	34.56	2764.48	21752.2	8834	UNPLUGGED	
15	19553	6023	39.86	3188.48	23589.9	10008	UNPLUGGED	
16	20810	6700	45.51	3640.72	25560.8	11951	UNPLUGGED	
17	22067	7411	51.51	4121.04	27617.9	13842	UNPLUGGED	
18	23323	8151	57.97	4633.06	29859.4	15896	UNPLUGGED	
19	24580	8931	64.58	5176.24	32300.5	18095	UNPLUGGED	
20	25837	9741	71.64	5750.22	34938.5	20439	UNPLUGGED	
21	27094	10582	79.17	6354.56	37677.6	22937	UNPLUGGED	
22	28351	11463	87.24	6989.84	40522.8	25589	UNPLUGGED	
23	29608	12384	95.84	7655.68	43479.2	28394	UNPLUGGED	
24	30865	13345	104.97	8352.6	46550.8	31354	UNPLUGGED	
25	32122	14347	114.64	9080.4	49742.6	34470	UNPLUGGED	
26	33379	15390	124.94	9838.8	53059.6	37744	UNPLUGGED	
27	34636	16474	135.87	10627.4	56507.0	41179	UNPLUGGED	
28	35893	17600	147.42	11446.8	60090.0	44784	UNPLUGGED	
29	37150	18768	159.59	12297.6	63814.4	49474	UNPLUGGED	
30	38407	20000	172.36	13180.4	67685.2	54274	UNPLUGGED	
31	39664	21300	185.74	14094.8	71708.4	59199	UNPLUGGED	
32	40921	22670	199.74	15040.4	75890.8	64254	UNPLUGGED	
33	42178	24110	214.36	16016.8	80228.4	69544	UNPLUGGED	
34	43435	25620	229.61	17024.8	84727.2	75074	UNPLUGGED	
35	44692	27200	245.49	18064.0	89392.0	80854	UNPLUGGED	
36	45949	28850	261.99	19134.8	94219.2	86884	UNPLUGGED	
37	47206	30570	279.11	20236.8	99216.0	93164	UNPLUGGED	
38	48463	32370	296.84	21370.4	104380.8	99704	UNPLUGGED	
39	49720	34240	315.18	22536.0	109711.2	106514	UNPLUGGED	
40	50977	36180	334.14	23733.6	115216.8	113644	UNPLUGGED	
41	52234	38190	353.62	24964.0	120897.6	121104	UNPLUGGED	
42	53491	40270	373.64	26227.2	126753.6	128994	UNPLUGGED	
43	54748	42420	394.20	27523.2	132784.8	137224	UNPLUGGED	
44	56005	44640	415.32	28852.8	139001.6	145804	UNPLUGGED	
45	57262	46930	437.00	30216.0	145404.0	154744	UNPLUGGED	
46	58519	49290	459.24	31612.8	152001.6	164064	UNPLUGGED	
47	59776	51720	482.04	33043.2	158804.8	173774	UNPLUGGED	
48	61033	54220	505.40	34507.2	165824.0	183894	UNPLUGGED	
49	62290	56790	529.32	36004.8	173057.6	194424	UNPLUGGED	
50	63547	59430	553.80	37536.0	180505.6	205464	UNPLUGGED	

Total Capacity Plugged (kN) Total Capacity Unplugged (kN) Factored Total Capacity (kN) Utilisation Hammer Type/Size

Appendix C

Piling Driveability Assessment Results

PILE DRIVING ASSESSMENT

Pile Driveability Calculations



Client : Seagreen

Project : Round 3 Firth of Forth Phase 1

Project No : C162

Revision History

Revision	Purpose	Date	Author	Checked	Reviewed
0	For Comment	06/03/2002	VTE	MRO	JIR

**C162 - Seagreen Round 3 Firth of Forth Phase 1 Development Zone
Additional Pile Driveability Assessment including variable efficiency and drive-drill drive mode
Pile Driving Mode Results**

Scenario	Pile Diameter	Required				Achieved Capacity				Utilisation				Max SRD (kN)				Hammer Size				Driveability Results				Total Blow Count		Duration*		Enthru (kJ)		Comment/Assumptions
		Comp (MN)		Ten (MN)		Required Pile Length (m)	Comp (MN)		Ten (MN)		Compress	Tens	Max SRD (kN)	Hammer Size	Driveable Pile Length (m)	Max Comp (MPa)	Max Ten (MPa)	Total Blow Count	Duration* (Hour)	Min (15%)	Max (95%)	Plugged at 24m	Unplugged driving	Plugged at 25m	Unplugged driving	Plugged at 30m	Unplugged driving	Unplugged driving	Unplugged driving			
		Comp (MN)	Ten (MN)	Comp (MN)	Ten (MN)		Comp (MN)	Ten (MN)	Max SRD (kN)	Hammer Size																				Driveable Pile Length (m)	Max Comp (MPa)	
MCB_BE	2m	34.07	25.97	51.80	27.10	0.66	0.96	78504	IHC-S1800	27	235.23	-24.10	2316	0.9	233	1420	Plugged at 24m															
MCB_BE	3m	34.07	25.97	55.97	26.52	0.61	0.98	61027	IHC-S1800	22	198.47	-31.81	1329	0.5	231	1436	Unplugged driving															
MCB_UB	2m	34.07	25.97	57.28	27.84	0.59	0.93	90766	IHC-S2300	25	248.68	-32.55	1553	0.6	327	2056	Plugged at 25m															
MCB_UB	3m	34.07	25.97	60.80	28.43	0.56	0.91	67648	IHC-S2300	21	211.21	-27.33	893	0.3	333	2081	Unplugged driving															
DCB_BE	2m	34.07	25.97	49.42	26.80	0.69	0.97	78083	IHC-S1800	32	233.78	-27.32	2381	0.9	233	1432	Plugged at 30m															
DCB_BE	3m	34.07	25.97	55.00	26.06	0.62	1.00	56647	IHC-S1800	27	196.36	-38.32	1406	0.5	231	1449	Unplugged driving															
DCB_UB	2m	34.07	25.97	56.15	27.18	0.61	0.96	58340	IHC-S1800	30	229.75	-27.07	1597	0.6	233	1445	Unplugged driving															
DCB_UB	3m	34.07	25.97	58.00	27.00	0.59	0.96	63156	IHC-S1800	26	196.98	-39.51	1393	0.5	231	1446	Unplugged driving															
DCB_UB_7MW	2m	38.71	29.51	61.79	31.93	0.63	0.92	66668	IHC-S2300	32			n/a				Refusal in plugged mode at 30m BSBL															
DCB_UB_7MW	3m	38.71	29.51	64.54	30.34	0.59	0.96	69190	IHC-S1800	27	197.50	-58.60	1421	0.5	231	1449	Unplugged driving															

*Does not include changes in hammer efficiency, helmets etc and assumes a continuous driving process at 45blows/min

C162 - Seagreen Round 3 Firth of Forth Phase 1 Development Zone

Additional Pile Driveability Assessment

Pile Driving Mode

Ground Model	Parameter Model	Pile Diameter (m)	Pile Length (m)	Hammer Model	Efficiency (%)	Depth(m)	SRD (kN)	Blows per meter	Compression (MPa)	Tension (MPa)	Energy (kJ)	Cumulative Blowcount
MCB	BE	2	27	IHC S1800	15%	0.0	2000	0	0	0	0	0
					15%	6.0	4000	26	86	-51	233	77
					15%	10.0	5000	33	86	-46	233	195
					15%	11.5	5200	35	86	-45	233	246
					35%	14.5	14539	72	122	-31	411	405
					55%	17.5	24205	80	155	-25	644	632
					75%	22.5	40649	87	203	-29	1143	1047
					95%	23.5	70782	228	234	-26	1428	1205
					95%	25.5	74626	274	235	-25	1425	1707
					95%	27.0	78504	335	235	-24	1421	2316
MCB	BE	3	22	IHC S1800	15%	0.0	470	0	0	0	0	0
					15%	1.5	1377	0	0	0	0	0
					15%	3.5	3031	21	73	-57	231	21
					15%	6.1	4915	33	74	-51	231	90
					15%	8.5	7827	55	74	-42	230	196
					35%	11.5	17552	64	116	-37	537	375
					55%	13.6	31502	81	148	-25	840	527
					75%	16.5	46152	92	174	-23	1139	778
					95%	19.5	51085	84	197	-27	1441	1042
					95%	22.0	61027	107	198	-32	1436	1329
MCB	UB	2	25	IHC S2300	15%	3.5	912	0	0	0	0	0
					15%	6.8	2723	14	88	-46	327	23
					15%	10.5	5504	26	89	-30	328	97
					15%	12.7	10575	53	89	-19	330	184
					35%	15.5	21337	53	138	-13	780	332
					35%	18.5	33084	93	139	-13	775	550
					55%	21.5	45055	96	177	-18	1214	833
					75%	23.5	53161	91	208	-23	1653	1019
					95%	24.5	88347	308	247	-33	2060	1219
					95%	25.0	90766	361	249	-33	2056	1553
MCB	UB	3	21	IHC S2300	15%	3.5	1377	0	0	0	0	0
					15%	6.1	3467	16	79	-49	333	21
					15%	7.5	4806	21	79	-44	333	48
					15%	9.5	7035	33	79	-37	333	102
					15%	12.1	10474	51	79	-26	333	211
					35%	13.6	21079	53	123	-13	778	289
					35%	15.5	32067	82	124	-17	780	416
					55%	17.5	43778	77	158	-25	1218	575
					75%	19.5	55638	79	186	-25	1653	731
					95%	21.0	67648	83	211	-27	2081	893
DCB	BE	2	32	IHC S1800	15%	7.5	2710.0	20	86	-65	233	75
					15%	10.5	4509.0	29	86	-58	232	149
					15%	13.5	6720.00	44	86	-48	230	258
					15%	16.5	9343	61	86	-38	229	415
					15%	19.5	12377	88	87	-29	224	639
					35%	22.5	21388	72	136	-35	530	879
					55%	25.5	31130	73	172	-29	829	1096
					75%	28.5	41060	88	202	-25	1144	1337
					95%	29.5	72083.0	247	233	-24	1436	1505
					95%	32.0	78083.0	337	234	-27	1432	2381
DCB	BE	3	27	IHC S1800	15%	3.5	1500	0	0	0	0	0
					15%	6.1	3031	21	73	-61	231	27
					15%	7.5	4082	28	74	-57	231	61
					15%	10.5	6788	48	74	-49	230	174
					15%	13.5	10110	74	74	-40	230	357
					35%	16.5	14050	51	116	-55	539	544
					35%	19.5	18607	71	116	-41	538	726
					55%	22.5	32134	82	147	-29	844	955
					75%	25.5	46747	93	173	-33	1146	1217
					95%	27.0	56647	96	196	-38	1449	1406
DCB	UB	2	30	IHC S1800	15%	5.3	1640	0	0	0	0	0
					15%	6.8	2453	19	86	-61	233	15
					15%	9.5	4303	29	86	-51	233	79
					15%	12.5	6878	48	86	-38	232	194
					15%	15.5	10007	73	86	-25	232	376
					35%	18.5	13691	50	134	-30	542	561
					35%	21.5	22418	84	135	-12	538	761
					75%	25.5	38070	81	201	-32	1147	1089
					95%	28.5	50138	100	229	-27	1448	1360
					95%	30.0	58340	137	230	-27	1445	1597
DCB	UB	3	26	IHC S1800	15%	5.3	2473	0	0	0	0	0
					15%	6.8	3697	25	73	-58	231	19
					15%	9.5	6478	46	74	-49	230	115
					15%	12.5	10347	75	74	-38	230	296
					15%	15.5	15047	55	116	-51	539	491
					35%	18.5	20580	81	116	-35	538	694
					35%	21.5	33689	85	147	-28	843	943
					75%	24.5	51228	84	196	-36	1449	1198
					95%	25.5	57168	97	197	-39	1448	1288
					95%	26.0	63155	112	197	-40	1446	1393
DCB_7MW	UB	2	32 (plugged)	IHC S2300	Refusal at approximately 30m below mudline							
DCB_7MW	UB	3	27	IHC S1800	15%	5.3	2473	0	0	0	0	0
					15%	6.8	3697	25	73	-59	231	19
					15%	9.5	6478	46	74	-50	230	115
					15%	12.5	10347	75	74	-39	230	297
					35%	15.5	15047	55	116	-52	539	492
					35%	18.5	20580	81	116	-35	538	695
					55%	21.5	33690	85	147	-30	843	945
					75%	23.5	45335	90	173	-32	1146	1120
					95%	25.5	57168	97	196	-38	1449	1307
					95%	26.5	69190	131	197	-37	1445	1421

C162 – Saagreen Round 3 Firth of Forth Phase 1 Development Zone
 Additional Pile Drivability Assessment including variable efficiency and drive-drill drive mode
 Drive Drill Drive Mode Results

Scenario	Pile Diameter	Required				Achieved Capacity				Utilisation				Axial Capacity							Pile Drivability Assessment										Comment
		Comp (MN)	Ten (MN)	Required Pile Length (m)	Comp (MN)	Ten (MN)	Compress	Tens	Max SRD (kN)	Hammer Size	Drilled section (in below mudline)	Max Comp (MPa)	Max Ten (MPa)	Total Blow Count	Duration (Min)	Duration* (Hour)	Enthru (k)														
																	Min (15%)	Max (75%)													
MCB_BE	2m	34.07	25.97	29	34.35	31.48	0.99	0.82	40837	IHC S1200	17	190.28	-63.58	1445	32	0.5	189	916	See report for assumptions adopted.												
MCB_BE	3m	34.07	25.97	24	35.74	31.75	0.95	0.82	40293	IHC S1200	12	161.02	-59.45	1362	30	0.5	186	920	See report for assumptions adopted.												
MCB_UB	2m	34.07	25.97	27	35.91	32.44	0.95	0.80	40773	IHC S1200	15	190.49	-64.65	1351	30	0.5	189	915	See report for assumptions adopted.												
MCB_UB	3m	34.07	25.97	22	36.74	31.64	0.93	0.82	48956	IHC S1200	10	161.36	-58.78	1081	24	0.4	187	919	See report for assumptions adopted.												
DCB_BE	2m	34.07	25.97	35	35.20	32.60	0.97	0.80	39397	IHC S1200	15	182.74	-54.74	2061	46	0.8	176	870	See report for assumptions adopted.												
DCB_BE	3m	34.07	25.97	29	35.56	31.57	0.96	0.82	46347	IHC S1200	9	154.81	-54.69	1746	39	0.6	175	870	See report for assumptions adopted.												
DCB_UB	2m	34.07	25.97	32	35.40	31.90	0.96	0.81	40323	IHC S1200	12	182.90	-58.91	1802	40	0.7	177	869	See report for assumptions adopted.												
DCB_UB	3m	34.07	25.97	27	35.41	30.34	0.96	0.86	47722	IHC S1200	7	154.90	-58.09	1662	37	0.6	175	869	See report for assumptions adopted.												
DCB_UB_7MW	2m	38.71	29.51	34	40.35	36.80	0.96	0.80	45580	IHC S1200	14	206.89	-50.39	2062	46	0.8	176	1100	Max energy quoted is at 95% efficiency												
DCB_UB_7MW	3m	38.71	29.51	29	42.28	37.13	0.92	0.79	55619	IHC S1200	9	175.19	-53.54	2199	49	0.8	175	1099	Max energy quoted is at 95% efficiency												

C162 - Seagreen Round 3 Firth of Forth Phase 1 Development Zone

Additional Pile Driveability Assessment

Drive Drill Drive Mode

Ground Model	Parameter Model	Pile Diameter (m)	Pile Length (m)	Hammer Model	Efficiency (%)	Depth(m)	SRD (kN)	Blows per meter	Compression (MPa)	Tension (MPa)	Energy (kJ)	Cumulative Blowcount
MCB	BE	2	29	IHC S1200	15%	7.5	3263.9	24	80	-64	189	89
					15%	9.5	4509.4	34	80	-59	188	146
					15%	11.5	5632	43	80	-55	188	223
					35%	12.7	14079	52	126	-59	435	280
					35%	14.5	11345	41	126	-68	437	364
					35%	17.5	16676	65	126	-51	434	524
					35%	20.5	22311	85	126	-40	431	750
					55%	23.5	28227	72	161	-43	677	985
					55%	26.5	34407	90	162	-35	673	1228
					75%	29.0	40837	84	190	-32	916	1445
MCB	BE	3	24	IHC S1200	15%	6.8	3533	29	68	-59	186	97
					15%	9.5	5818	47	68	-54	186	199
					15%	11.5	7827	66	68	-50	186	313
					35%	12.7	18417	91	107	-52	432	407
					35%	14.5	21660	71	107	-60	433	553
					35%	16.5	17765	96	107	-49	431	719
					55%	18.5	23206	79	136	-59	679	894
					55%	20.5	28780	90	136	-50	677	1063
					75%	22.5	34478	80	161	-53	922	1233
					75%	24.0	40293	91	161	-50	920	1362
MCB	UB	2	27	IHC S1200	15%	6.8	2723	21	80	-65	189	70
					15%	9.5	4671	35	80	-57	188	146
					15%	12.1	6958	54	80	-48	187	261
					35%	13.6	16779	66	126	-44	433	351
					35%	14.5	11447	42	126	-64	437	399
					35%	16.5	15494	59	126	-49	434	500
					35%	19.5	21904	85	127	-35	431	717
					55%	22.5	28691	75	161	-40	675	957
					55%	25.5	35829	97	162	-34	671	1215
					75%	27.0	40773	85	190	-35	915	1351
MCB	UB	3	22	IHC S1200	15%	6.1	3467	26	68	-59	187	78
					15%	9.5	7035	57	68	-50	186	270
					15%	11.5	9633	79	68	-45	185	455
					15%	12.1	10473	86	68	-43	185	519
					55%	13.6	30400	83	136	-47	678	564
					55%	15.5	24200	67	136	-62	679	515
					55%	17.5	30939	84	136	-46	678	736
					55%	19.5	37953	100	137	-44	676	974
					75%	21.5	45226	91	161	-47	920	975
					75%	22.0	48956	98	161	-47	919	1081
DCB	BE	2	35	IHC S1200	15%	12.5	5938	50	77	-55	176	313
					15%	17.5	10309	92	78	-42	175	667
					35%	19.5	12377	49	121	-67	411	808
					35%	21.5	20760	82	122	-48	408	939
					35%	23.5	15874	67	122	-59	409	1088
					35%	25.5	19647	79	122	-50	408	1234
					35%	27.5	23483	93	122	-42	407	1406
					55%	30.5	29348	78	155	-47	640	1662
					55%	33.5	35338	98	155	-35	638	1925
					75%	35.0	39397	83	183	-39	870	2061
DCB	BE	3	29	IHC S1200	15%	9.5	5818	51	65	-55	175	244
					15%	12.5	8935	81	65	-49	174	442
					35%	15.5	12669	54	103	-74	409	645
					35%	18.5	17020	73	103	-64	408	836
					35%	19.5	18607	81	103	-61	408	914
					55%	21.5	36391	98	131	-51	639	1092
					55%	23.5	27862	79	131	-64	641	1269
					55%	25.5	33928	92	131	-53	639	1440
					75%	27.5	40091	82	155	-56	871	1614
					75%	29.0	46347	94	155	-47	870	1746
DCB	UB	2	32	IHC S1200	15%	9.5	4303	35	77	-59	177	164
					15%	12.5	6878	57	77	-49	176	301
					15%	15.5	10007	87	78	-39	175	516
					35%	18.5	13691	55	122	-57	410	728
					35%	19.5	15042	62	122	-53	409	787
					35%	21.5	24239	98	122	-38	407	946
					35%	23.5	18554	77	122	-45	408	1121
					55%	26.5	25456	69	155	-47	640	1340
					55%	29.5	32717	90	155	-34	638	1580
					75%	32.0	40323	88	183	-32	869	1802
DCB	UB	3	27	IHC S1200	15%	6.8	3697	31	65	-58	175	106
					15%	9.5	6478	57	65	-53	175	225
					15%	12.5	10347	94	65	-45	174	452
					35%	15.5	15047	64	103	-66	408	688
					35%	18.5	20580	93	103	-53	407	924
					35%	19.5	22608	99	103	-49	407	1020
					75%	21.5	42590	89	155	-48	870	1208
					75%	23.5	32677	71	154	-63	873	1368
					75%	25.5	40072	84	155	-50	871	1524
					75%	27.0	47722	100	155	-43	869	1662
DCB_7MW	UB	2	34	IHC S1200	15%	12.5	6878	57	77	-50	176	356
					15%	15.5	10007	88	78	-41	175	572
					35%	18.5	13691	55	121	-61	410	786
					35%	19.5	15042	62	122	-58	410	845
					35%	21.5	24239	97	122	-40	407	1004
					35%	23.5	18554	76	122	-49	408	1177
					55%	26.5	25456	68	155	-51	641	1393
					55%	29.5	32717	89	155	-36	638	1629
					75%	32.5	40323	87	183	-33	869	1893
					95%	34.5	45580	82	207	-35	1100	2062
DCB_7MW	UB	3	29	IHC S1200	15%	12.5	6478	58	65	-54	175	361
					15%	15.5	10347	94	65	-46	174	589
					35%	18.5	15047	64	103	-68	408	827
					35%	19.5	20580	92	103	-57	407	905
					35%	21.5	22608	97	103	-53	407	1094
					75%	23.5	42590	87	155	-53	871	1278
					75%	26.5	32677	70	154	-71	873	1512
					75%	29.5	40072	82	155	-56	871	1740
					75%	32.5	47722	97	155	-45	870	2009
					95%	34.5	55619	93	175	-44	1099	2199