

## Aberdeen Harbour Expansion Project

**Construction Environmental Management Document** 

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# Chapter 3 Construction Method Statement



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#### 1. PURPOSE

This Construction Method Statement (CMS), prepared by Dragados UK, details the construction methodology for the Aberdeen Harbour Expansion Project (AHEP). It has been produced to inform stakeholders regarding the procedures and programme of the AHEP construction phase.



Figure 1- Aerial view of the bay prior to construction



#### 2. CONSTRUCTION WORKS

#### 2.1 DESCRIPTION OF WORKS

The main activities and work packages for the construction of AHEP include:

- Dredging the existing bay to design depths varying from -9 to -15.25m chart datum (CD). The dredged material is expected to comprise of sand/alluvium, glacial till and rock materials.
- Profiling the existing Southern slopes of the bay. This is intended to reduce wave reflection within the central berthing and approach channel areas of the development by absorbing incoming waves.
- Construction of two rubble mound breakwaters 640 metres (North Breakwater) and 605 metres (South breakwater) long. The purpose of these structures is to protect the new facilities from the North Sea met ocean conditions.
- Construction of approximately 897m of closed and 535m of open quays¹ to provide a combined total of over 1432m of quayside capable of berthing vessels.
- Land reclamation activities to provide a paved area immediately to the rear of the quayside installations. This will use materials recovered from dredging operations supplemented by imported materials.
- Provision of ancillary welfare accommodation, quayside furniture and water tank installations for the facilities operational stages.
- Numerous stages of off-site highway work to allow free flowing traffic around the new facilities during construction and operation. This will include improved access for Heavy Goods Vehicles (HGVs).

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<sup>&</sup>lt;sup>1</sup> Closed quays have a foundation with a solid construction, which presents a solid barrier to seawater under the deck. An open quay has a deck supported on piles, which allows seawater to pass through the underside of the deck.



Figure 2 - Imposed aerial view of the bay after completion of construction

#### 2.2 WORK COMPLETED

The following works have been completed in 2017,2018 and 2019:

- · Completion of the enabling works and surveys;
- Installation of temporary perimeter fencing to secure the site boundaries and earth works in the compound area;
- Construction of the temporary access routes;
- Preparation of Central and Southern Compounds for welfare and pre cast yard installations respectively;
- · Preparation of site offices in the Central compound;
- Installation of temporary visitor centre in the Northern Compound, as well as the Precast Concrete Blocks casting & storage areas.
- Fabrication of precast elements such as accropodes has begun. 6,408 units now fabricated;
- Coast Road Widening Works;
- St Fitticks Junction Road Works in progress up to 1<sup>st</sup> phase of the traffic management (one side of St Fitticks Road is completed).
- Installation of Construction Phase SUDS (described in Section 5.1.3.1 to 5.1.3.3);
- Temporary and Permanent Coastal/Cycle Path Construction and Alignment (Appendix 2);
- Construction of full length of North Breakwater (remedial works are still ongoing on limited areas);

- Construction of 80 metres of the SBW, including its access and its winter protection;
- · Open Quay. 120 Piles performed at October 2019;
- Close Quay. 22 Caissons built in Spain and all have been shipped to Scotland. As of October 2019, 11 caissons have been installed at Aberdeen Harbour Expansion Project and 11 caissons are in storage in Cromarty;
- Caisson Infill, Backfill, and Closed Quay general fill have been performed on the areas where caissons have been placed. Progress has been sufficiently relevant in the East Quay, up to the point to commission a temporary quay on that area.
- Dredging and offshore deposit of approximately 2,150,000 m<sup>3</sup> of soft material using a trailer suction hopper dredger and backhoe vessels between 2017, 2018 and 2019;
- Drilling and Blasting approximately 45,000 m3 of rock in the North Quay
- Additionally, Rock removal by mechanical means has been performed, reaching in total (including blasted volume) 153,500 m3 of rock in the North Quay area and Southern areas.



Figure 3 - Aerial view of of the AHEP



#### 3. SITE LAYOUT

#### 3.1 SITE ESTABLISHMENT AND WELFARE

There are three temporary compounds being used for the duration of the construction period:

- 1) Central Compound: area surrounding the junction of St Fitticks Road, Coast Road and Greyhope Road previously used for recreational purposes. The Central Compound hosts the site offices (for management and engineering staff) and project welfare facilities (for operatives and skilled labour). Wheel-cleaning facilities are available at the entrance through to NBW.
- 2) North Compound: area located in the existing Walker Park. The North Compound houses the visitor centre, and acts as a storage area for the construction material, if required.
- 3) South Compound: area located south of Nigg Bay previously used for agricultural purposes. This area contains the precast and accropode fabrication plant and storage, laboratory, batching plant, two boreholes supplying water to the batching plant, temporary access to the storage facilities, as well as access to the Southern Breakwater.

The location of the compounds in relation to the main construction activities means the site is accessible and serviced by one of the three compound areas without the need for a shuttle or mobile welfare services. Wheel-cleaning facilities have been implemented at the exit points at the central and south compound to prevent dirt and debris from leaving the work areas.

See Appendix 1 (A, B and C) for proposed plans of the 3 construction compounds.

#### 3.2 ACCESS TO THE SITE

AHEP is accessible by several routes for light goods and personal vehicles travelling from the city centre and South. The site is limited to one single route from the south for HGV traffic.

The site is signposted with all relevant restrictions shown in advance though a Traffic Management Plan. This includes the installation of semi-permanent traffic signs on poles and a dedicated customer enquiries telephone line.

Further information is provided in the Construction Traffic Management Plan (Chapter 6 of the CEMD).

#### 3.2.1 Access for LGV

Light goods vehicles (LGVs) and cars are permitted to access the site without restriction. If travelling from the South the route in Section 3.2.2 should be followed. If travelling from the North or City Centre then the route detailed below and in Figure 4 should be followed:

- Cross the A956 road bridge over the River Dee and take the first exit from the Roundabout onto South Esplanade West.
- Follow South Esplanade West until its Junction with Victoria Road under traffic signal



• Turn right at the traffic signals and follow Victoria Road in a South easterly direction. Follow signage for desired site access point.

#### 3.2.2 Access for HGV

HGVs are restricted to one point of access to and from the site only. This access route has been agreed and documented with regulatory bodies and <u>MUST</u> be adhered to at all times. Under no circumstances will any HGV access the site by any other means unless fully authorised by the site team and appropriate regulatory authorities.

The route to be followed is as detailed in Figure 4.

- Travel East on Hareness Road from the A956
- · Continue East leaving 2No roundabouts at their second exits
- At the T-Junction between Hareness and Coast Road, turn North (left)
- Follow Coast Road in a northerly direction passing over a railway bridge controlled by traffic signals. NOTE THIS STRUCTURE HAS A WEIGHT LIMIT OF 40T
- On leaving the railway bridge, follow signage for desired site access points (section 3.3)



Figure 4 - Designated access route for HGVs (blue), cars and LGVs (green) to access Nigg Bay

#### 3.2.3 Delivery of Stone

All HGVs transporting rock are required to follow the traffic access route to Nigg Bay as outlined in the Construction Traffic Management Plan (Chapter 6 of the CEMD).



#### 3.3 ACCESS TO CONSTRUCTION AREAS

Access within the site is provided by several temporary access routes, each serviced either by Coast Road or Greyhope Road.

The temporary access routes (Table 1) will be formed of a granular material and will be maintained to ensure that maximum accessibility is available. The routes form an additional 5 access points for the site to those listed in 3.2. Illustrative maps of access routes within the three Construction Compound are provided within Section 6.4 of the Construction Traffic Management Plan, which can be found in Chapter 6 of this CEMD.

Table 1 - Description of temporary access routes.

Termination Point	Information
South Breakwater	Access only from the Southern Compound. HGVs will use this route travelling from the south to deliver to the southern breakwater.
Fabrication Yard	Access to the fabrication facilities for all vehicles. Access to the southern breakwater for vehicles traveling from the north.
Southern Slopes (Land Blasting)	Access for land blasting equipment to profile the Southern slopes which may be not practical to complete from the marine environment.
North Breakwater	Access for HGVs and general traffic from the south, delivering material or requiring access to the North Breakwater.
Project Offices	Accessible from Greyhope Road which will be closed at its junction with St. Fitticks. This access route will allow traffic to enter the project's office management facilities.



#### 4. PROGRAMME OF WORKS (AS OF OCTOBER 2019)

The programme duration of 177 weeks began when contract was awarded on the 20<sup>th</sup> December 2016. The programme is provided in Appendix 3 and a description of remaining work has been summarised below. A description of completed work has been summarised in section 2.2.

Revised completion date is June 2020 due to unforeseeable extreme weather events which occurred at the start of 2018 (new duration of 181 weeks).

Planned completion date is in December 2021. Additional slippage is caused but not limited to effects of additional weather events (which have not still been considered).

The first phase of marine construction activities was undertaken from May to November 2017. The initial emphasis was focused on the north breakwater including placement of core breakwater material and direct deposit of quarry material.

The north breakwater activities ceased at the end of December 2018 for storm protection.

In early 2018, the caisson fabrication commenced in mainland Spain. Preparatory and fabrication works were undertaken from February 2018 to July 2018. The first 15no Caissons were delivered in September and October 2018. The following 7no caissons were transported from Spain to Cromarty Firth in July 2019 following extensive checks for non-native species.

The second phase of marine construction occurred from March to late 2019. In this phase the north breakwater has been completed. South breakwater activity will resume in 2020 and will be built with the blasted rock. Due to delays regarding planned blasting activities progress at the South Breakwater has been postponed.

The north breakwater provided shelter in Nigg Bay enabling quay installations to begin. Therefore, 11no caissons out of a total of 22no caissons have been placed in the East & North Quays.

South East Pier & relevant civil works of the Open Quays are limited by the progress of the South Breakwater (due to sheltering reasons as well).

Quay installation started in March 2019. Dredging activities continued from May 2019 with some of the dredge material used to in fill the caissons and behind the quays. Drainage, civil works and pavement installations complete the closed quay structure ready for building, furniture and installations.

Quayside activities and finishing works will complete the programme. This includes the installation of buildings and mechanical and electrical works which run from mid-2020. This will allow a project handover to Aberdeen Harbour Board in 2021.

These activities will be followed by the installation by Aberdeen Harbour Board of navigation aids, lights, CCTV and security.

#### 4.1 WORKING HOURS

The construction programme takes into account realistic working hours and third party restrictions.

Proposed working hours are detailed below, with general activities (any activity not listed within Table 2) being run 6 days per week from 07:00 to 19:00 Monday to Friday, and 09:00 to 16:00 on Saturdays with no Sunday working. As construction works progress, there is the potential for these working hours to be amended depending on progress.



Table 2 - Representation of construction working hours

<b>Work Aspect</b>	Activity	Working Hours	
Dredging/	Blasting	During daylight hours	
Blasting/	Drilling associated with blasting	24 hours, 7 days a week	
Drilling	Dredging	24 hours, 7 days a week	
Breakwater			
	Direct placement (from land)	24 hours, 7 days a week	
	Rock and Accropode armour placement	24 hours, 7 days a week	
	Crown Wall	24 hours, 7 days a week	
Quays	Revetment bedding layer and cells filling	24 hours, 7 days a week	
	Rotary Piling	24 hours, 7 days a week	
	Superstructure	16 hours, 7 days a week	
Casting yard	Precast Fabrication	24 hours, 7 days a week	
Caissons	Transport, placement and backfilling	24 hours, 7 days a week	



#### 5. CONSTRUCTION ACTIVITIES

#### 5.1 ENABLING WORKS

#### 5.1.1 Road Works

There are two main roadwork activities proposed (see Appendix 2 for plans):

- Works to the existing Coast Road to the South of AHEP, requiring the existing pavement surface to be widened to accommodate an increase in HGV traffic between Hareness Road and Doonies Farm.
- Realignment works to move the existing Coast, Greyhope and St. Fitticks Road(s) profiles to accommodate harbour entrance facilities.

#### 5.1.1.1 Widening Works to Coast Road [COMPLETED]

Pre-construction, traffic management was put in place and Coast Road closed, allowing safe and non-restricted access to the area for contractor(s). Site clearance works were carried out, such as removal of boundary wall/fence, signpost, signage and other street furniture. Following clearance activities, earthwork activities were completed involving the removal and importation of material(s) to form required profiles.

Once levelling occurred, the existing blacktop surface was removed through milling. The milling machine ran the route removing the required quantities of blacktop surface. The output material was discharged directly into the back of a truck for removal from site for recycling.

Once the road surface was reduced to required levels, blacktop components were delivered via truck to the location. The material was placed into the hopper of a paver and installed in the required thickness along the route. Following the paver, a roller compacted the material. The process was continued until the pavement is complete in each individual characteristic layer.

On completion of the running layer, line markings and surface textures were undertaken, signage reinstated, traffic management removed and the Coast Road opened to the public.

#### 5.1.1.2 Road Realignment to Coast, Greyhope and St. Fitticks Roads

Pre-construction, traffic management have been in place to allow safe working areas for the workforce and public around the realignment works. Site clearance works have been carried out including stripping of topsoil and furniture removal. All material will be stockpiled locally or managed according to the Waste Management Plan. Then earthwork will be completed including the removal and importation of material(s) to form required profiles. Milling activities will take place on existing carriageway tie in locations to form required profiles for the running surface transition.

Once the road surface has reached desired levels, blacktop will be placed at the required thickness along the route. Following the paver, a roller will compact the material. The process will be continued until the pavement is complete. Line markings and surface textures will be installed. Running simultaneously, boundary restraints and signage will be reinstated for full transition of traffic management to align vehicles to the new carriageway.



Some existing services along the Coast Road and St Fitticks Road have been diverted to enable the existing junction re-alignment. Trial pits have been dug to determine the exact location of the existing services. The new cable trench will then be excavated, new services laid and new connections made by a specialised statutory services provider. The new lines will be tested and commissioned before they are brought into service.

#### **5.1.2 Preparation of Contractors Working Areas [COMPLETED]**

Prior to construction, boundary fencing was installed around the working area. Depending on ground conditions, the fencing was installed by mechanical or hand excavation means at post locations and secured with a concrete/post mix. Site access points (ie entrances) were installed by excavators and trucks removing/importing material to form required ground profiles.

Within the site boundary, excavators and bulldozers profiled the land and installed drainage. Bulldozers pushed all material to a local point where excavators loaded for removal future storage and reuse. Where the land was not workable by a bulldozer, a mechanical excavator broke out hard material and loaded directly to a truck. Mechanical excavators locally formed all service trenches.

On reaching required ground levels, imported material was placed by truck for spreading via bulldozer. The bulldozer was followed by a roller to compact the material to the required bearing pressure(s). Services were installed and connected with local backfill completed by excavator.

Areas requiring higher bearing capacity were completed and finished through concrete slabs. A blinding layer was installed consisting of mass concrete mix delivered via a concrete truck and placed via excavator bucket. The concrete pour was completed using direct discharge where possible. All concrete trucks were sent back to the batching plant for washout.

The temporary office and welfare cabins are modular. These were installed using a Hiab or a 40 Ton crane on cast in situ footings. The cabins were then fitted out before brought into use.

The fabrication shed compromises a steel structure with frames at 5m spaced PVC coated sandwich façade and roof and cast in situ footings. This was erected using a Hiab or a 40 Ton crane.

The 3 construction compounds required the following connections to the existing services:

- Northern Compound: A septic tank has been installed at the Visitor Centre which will be emptied by a Licensed Waste carrier. Aberdeen City Council have advised that no drainage is required on the access road as there is sufficient camber on the road to allow rain water to percolate into the surrounding ground. The power supply will be provided by the local distribution operator (SSE) using an existing distribution route along Greyhope Road.
- Central Compound: A connection has been made to the St Fitticks sewer. The power supply was provided by SSE using an existing distribution route along St Fitticks Road. The water supply has been provided by SW using an existing distribution route along St Fitticks Road.
- **Southern Compound:** A foul and effluent discharge into the existing 900mm combined sewer has been connected. The power supply was provided by SSE using an existing



distribution route along the Coast Road. Rainwater is being collected by the compound drainage system and discharged into the sea, via settlement ponds.

#### **5.1.3 Installation of Construction Phase SUDS [COMPLETED]**

Sections 5.1.3.1. to 5.1.3.3. list the actions that will be taken to install the construction phase SUDS across the Northern, Central and Southern Compounds at the AHEP site.

#### 5.1.3.1 Northern Compound

The following sequence of works are anticipated within the northern compound:

- 1. Establish compound boundary fences and compound site access point;
- 2. Topsoil strip and stockpile;
- 3. Bulk earthworks to formation levels:
- 4. Installation of Temporary Septic Tank and Water Tank;
- 5. Construction of on-site roads and formation of storage areas; and
- 6. Construction of Visitor Centre

#### 5.1.3.2 Central Compound

The following sequence of works are anticipated within the Central compound:

- 1. Establish boundary fences and temporary site access points;
- 2. Install surface water drainage connection to Girdleness sewer; (Completed)
- 3. Installation of wheel cleaning facilities;
- 4. Topsoil strip and stockpile;
- 5. Bulk earthworks to formation levels;
- 6. Installation of "permanent" drainage (surface water and foul);
- Construction of on-site roads and formation of storage areas and completion of compound permanent protection measures to drainage features (channels, gullies and connections); and
- 8. Construction of Offices.
- 9. Connection with SSE power supply network (Completed)
- 10. Connection with SW supply network (Completed)

#### 5.1.3.3 Southern Compound

The following sequence of works are anticipated within the southern compound:

- 1. Establish boundary fences and temporary site access points;
- Installation of 2 permanent stilling ponds and perimeter stockpile drainage. This is a
  cut -off ditch that will collect the run-off and convey to the stilling ponds preventing
  any silty water from leaving the site area. This arrangement will be in place for the
  duration of the construction phase;



- 3. Install surface water drainage outfall in Nigg Bay;
- 4. Installation of wheel cleaning facilities;
- 5. Topsoil strip and stockpile;
- 6. Installation of temporary cut off drains installed around the perimeter of each earthworks section prior to each earthwork phase;
- 7. Bulk earthworks to formation levels;
- 8. Installation of "permanent" drainage (surface water and foul);
- 9. Construction of on-site roads, formation of storage areas and final surfacing and completion of compound permanent protection measures to drainage features (channels, gullies and connections); and
- 10. Construction of accropode fabrication buildings.

During the first stages of the works (earthworks) a temporary drainage system will be installed. This will consist of a temporary trench 50cm deep and a temporary stockpile of material of 50cm on the external side of the temporary trench. This will discharge into two stilling ponds located at the lowest point of each compound area.

This temporary arrangement will be in place until the permanent compound drainage system is completed.

#### 5.1.4 Temporary and Permanent Coastal/Cycle Route Alignment

A primary network of paths borders the site location, consisting of the National Cycle Network and Coastal Path. The paths have been developed so that no section of the network will be closed before a new section is open to the public. Both the temporary and permanent makeup of paths are similar in nature and construction.

The layout of the cycle path can be found in Appendix 2. The approach to management of vehicles and cyclist/pedestrians is described in the Construction Traffic Management Plan.

#### 5.1.4.1 Cycle Path Construction [COMPLETED]

Path installation was completed outside the current cycle route and independently fenced from surrounding activities. Where works were required on the existing cycle route, a segregation system was introduced to allow half of the path width to be worked on while the other remained open.

Excavation and fill profiles were completed using a small excavator and dump truck. The dump truck transported a local stockpile in manageable volumes to be placed as required to form the permanent works. Likewise, manageable volumes were removed in reverse order. On achieving required land profile, similar techniques were used to place path make up. A roller was then used to provide the required bearing pressure.

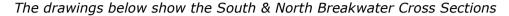
#### 5.2 BREAKWATER CONSTRUCTION

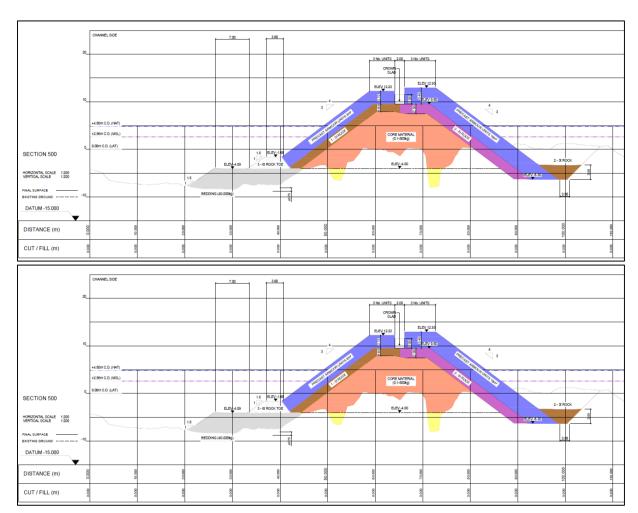
Two breakwaters will be built to protect the harbours inner basin from the North Sea metocean conditions.

At 1245m combined length, the breakwaters consist of core material / rock dredge material, secondary protection and accropodes. Core material will be placed on the seabed using



quarry material sized between 0.1-500kg. Secondary protection will be provided by placement of a heavier grade of rock sized between 1,000-5,000kg and toe berm rock sized between 2,000kg-5,000kg 3,000kg-6,000kg us. The final layer of protection is provided by 8-16m<sub>3</sub> accropode units. The North breakwater will also include a crown wall system.





Design levels show that the following quantities of material are required:

Table 3 – Indicative Quantities of material required for breakwater construction

	Unit of Measure	Quantities	
		North BW	South BW
Core Material/rock dredge material	Cubic metre	241,379	344,416
Bedding Material	Tonnes	11,237	22,504
Secondary Protection	Tonnes	52,316	101,671
8m <sup>3</sup> Accropode	Units	3,110	2,296
12m <sup>3</sup> Accropode	Units		1,839
16m <sup>3</sup> Accropode	Units		1,416



Completion of construction of the full length of the North Breakwater allowed start of the Crownwall construction works.

South Breakwater access and the initial 80 meters have been built in years 2018 and 2019. The works will continue after the winter period in 2019, once marine blasting operations start without constrains (except noise limits).



#### 5.2.1 Rock Trench Formation

Accropode units provide protection to the breakwaters. In order to stabilise the accropodes, formation of a rock trench is provided for the breakwaters outer footprint. As shown in Figure 5, the trench reacts by "locking in" the bottom accropode layers which support those above. This toe will be created by drilling & blasting techniques to allow excavation and / or the use of rockwheel, trepano, hydraulic ripper, expansives or other mechanical means at shallow areas.

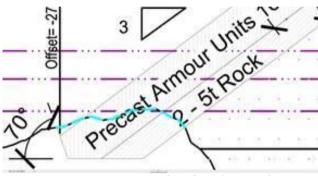


Figure 5 - Example of rock trench

#### 5.2.2 Core Material Placement

Two separate methods of installation will be used to deal with the different bathymetric features of the construction location as follows:

- in shallower waters core material will be installed by direct placement by road going vehicles (see section 5.2.3), and
- in deeper waters core materials will initially be installed by marine placement to levels which then become suitable for direct placement by road (see section 5.2.4).

#### 5.2.3 Direct Placement of Core Material

Dump trucks and quarry vehicles access the breakwater/land interface using the Breakwaters temporary access routes. These vehicles progress to discharge material at varying locations dependant on the breakwater progression, from the land outwards towards the breakwater head. The material is pushed by a bulldozer or excavator to the required location. Following this a long reach excavator grades the core material to form the required design slopes for which allow installation of further material.

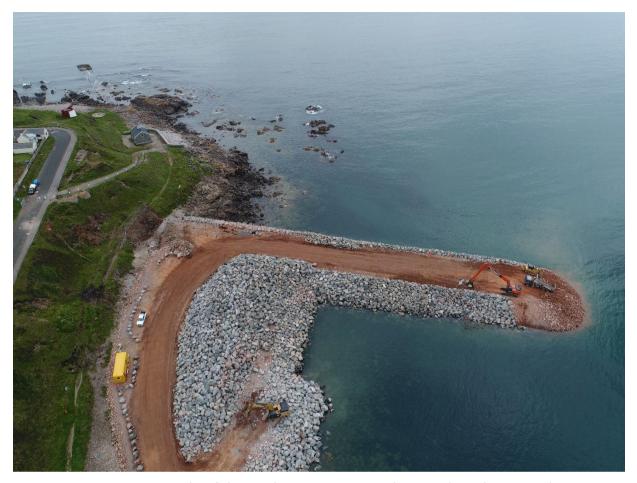


Figure 6 - Example of direct placement approach carried out by Dragados

#### 5.2.4 Marine Placement of Core Material

Marine placement increases bed levels to those reachable and manageable by direct placement (see section 5.2.3 above).

The marine placement of core material will be achieved by using a split hopper barge (see Figure 7). The barge will be loaded with core material from an auxiliary quay located beside the North breakwater of the site via dump trucks and quarry vehicles, or transferred ship to ship from Norway vessels or directly from the dredging operation. This auxiliary quay will be built with a similar method to the breakwaters. Once fully loaded, the barge will then position itself in the required location before opening its hull and releasing the core material into position. The barge will work from the land outwards.

Rock material from the dredging operation will be placed by a split barge as core material during the construction of the breakwaters. Marine equipment (backhoe or similar) will then profile the core / rock dredge material to the required tolerance.



Figure 7 - Example of quarry vehicles loading a split barge hopper for future discharge

#### **5.2.5 Secondary Armour Placement**

The secondary armour placements runs in tandem with core material placement. A heavy duty crawler crane and / or excavator placed the secondary and toe<sup>2</sup> armour material. The secondary and toe armour ensure the previously placed and shaped core material is contained. The toe armour acts as a stability mechanism and rests over a bedding layer material.

When a crane is used, (Figure 8), orange peel grab(s)or rock skip(s) place the material on the seaward and harbour boundaries of the breakwaters.

In the first section of the NBW (170 m landwards), toe is built using 44 Tn concrete blocks already stockpiled on site. Placement of these blocks is done with a heavy crawler crane from the NBW platform.

Before placing the blocks, it's necessary to prepare the seabed to ensure the right stability of the blocks.

Seabed preparation consists of achieving a regular base for the blocks by pecking locally the existing rock, with mechanical equipment.

Access to the seabed preparation works is achieved either by building a temporary causeway seaside of the NBW or gaining access directly from the NBW platform.

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<sup>&</sup>lt;sup>2</sup> The toe armour is part of the secondary armour located at the base of the breakwater on the seaward side



Figure 8 - Example of armour placement using orange peel grab carried out by Dragados

#### 5.2.6 Armour Unit Placement (Accropodes)

The accropode units act as the primary layer of defence. They are installed by a heavy duty crane and / or excavator fitted with specialized lifting tackle. The accropode units are installed one at a time as they are delivered from the fabrication yard on a specialised heavy duty trailer. Positioned with advanced GPS or Echoscope equipment, the accropode will be installed in its final location.

Marine equipment like a jack-up or semisubmersible vessel will be used as well to enable the heavy duty crane and/or to place the deepest accropodes in the SBW.



Figure 9 - Placement of Accropode Units

#### 5.2.7 Construction of Crown Wall

The north breakwater crown wall will be constructed over two seasons in weather windows in 2019 and 2020. The coronation slab (a concrete slab on top of the breakwater) will be completed in the last phase.

#### 5.2.7.1 Coronation Slab

The coronation slab will be constructed by use of a traditional formwork system <sup>3</sup>. Manageable lengths of formwork will be constructed by concrete placed directly from a concrete truck/pump and finished to level. Successful curing of the pour will enable the formwork to be removed and constructed within the next section until the coronation slab is completed.

#### 5.2.7.2 Crown Wall

The crown wall will be completed with a specialised travelling formwork system adapted to suit the specific job. The formwork will use the coronation slab and previous crown wall pour as guide to travelling along the element structure. The formwork is self-sufficient and is semiautomatic in operation with personnel and tools situated within (see Figure 10). The concrete will be transported by truck and placed via pump or conveyor.

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<sup>&</sup>lt;sup>3</sup> Formwork is the term given to moulds in which concrete is poured

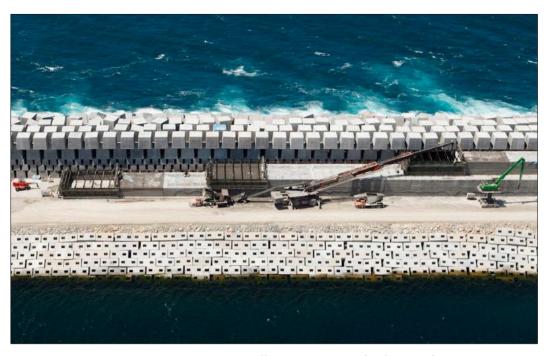


Figure 10 - Crown Wall Construction (indicative)



#### **5.3 DREDGING ACTIVITIES**

It is anticipated that dredging activities within the bay removes three main types of material: sand & alluvium, glacial till and rock. Removal and workability of each respective material requires a different method of removal in both approach and plant selection. Please see Dredging & Dredge Spoil Disposal Management Plan for further detailed information.

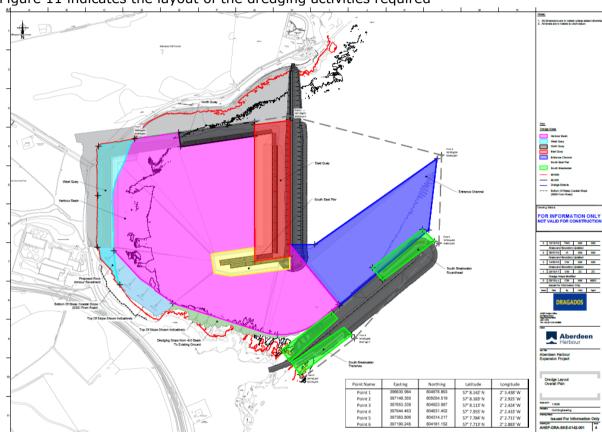


Figure 11 indicates the layout of the dredging activities required

Figure 11 - Layout of the Dredging Activities

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#### 5.3.1 Dredging Sand & Alluvium

Deposits of sand and alluvium are found within the top layers of the sea bed in Nigg Bay. Given their granular nature they are ideally removed by use of a backhoe dredger (BHD), trailing suction hopper dredge (TSHD) (see Figure 12) or possibly Cutter Suction Dredger (CSD).



Figure 12 - TSHD Draghead representation (Van Oord Volvox Terranova)

A self-propelled vessel, the TSHD will remove the granular material from the bed of the bay by trailing a draghead along the seabed. The draghead will be connected to the vessel by means of a suction pipe into the hull of the vessel. Comprising of several moving parts, the draghead will fit closely to the seabed contours. It uses a range of teeth and water jets to loosen the granular material. The loosened deposits are then vacuumed from the seabed up the suction pipe to the vessels hull by on-board sand pumps.

The material brought aboard the vessel will be stored locally in the vessels hull in a large compartment known as the hopper well. Emptying the material will carried out by either:

- Bottom discharge at an offshore deposit site The vessel will position over the deposit site (see section 5.3.4), and the bottom of the vessel will open up to discharge the stored material. Internal water jets will wash down the compartment before the bottom of the vessel is resealed for further dredging activity.
- Pumping onshore/into onshore vessel The vessel will connect to a floating coupling
  which directs a pipeline to the shore side/structure. Jets within the vessel compartment
  will add water and fluidise the dredge material. The fluidised material will then be
  offloaded from the vessel through this series of pipework. It is also possible to transfer
  materials by conveyor



#### 5.3.2 Dredging Glacial Till

Layers of glacial till can be found within the bay under the aforementioned layers of sand and alluvium. Due to its firm nature, many of the deposits are not fully recoverable by use of a TSHD, and best suited to a cutter suction dredger (CSD) or a backhoe dredger. (BHD).

A self-propelled vessel, the CSD is equipped with a rotating cutter head, for cutting and fragmenting hard material. The fragmented material is sucked up by means of dredge pumps, and discharged into split hopper barges that are moored alongside the CSD. These split hopper barges unload the soil at the offshore deposit site. During dredging the CSD vessel remains on station, secured by a 'spud' mooring lowered to the seabed. The dredger swings sideways by means of winches and anchors, and the cutter head cuts and removes the soil.

When a BHD is used the dredged material is placed in the hopper barges and disposed of at the offshore deposit site.

#### 5.3.3 Drum Cutter/ Rock Wheel

A drum cutter has been selected to sculpt areas of the harbour including trenches close to the breakwater and areas to the south of the bay. The use of blasting has been investigated in these areas however, it is unlikely that blasting will achieve the correct profile required and may result in over fracturing of material. At areas of shallow rock removal, an excavator mounted rock wheel will excavate out rock to level by grinding the rock to the required profile (Figure 13). This will likely occur at the east side of the north breakwater and at the west of the south breakwater. A drum cutter can also be used in more adverse weather conditions than blasting.



Figure 13 - Rock wheel

#### 5.3.4 Marine Drilling & Blasting

Specific details relating to marine and land blasting can be found in Chapter 7 the Dredge Dredging and Dredge Spoil Disposal Management and Monitoring Plan.

Below the sand/alluvium and glacial till layers, more dense and tough rock material can be found flowing down to the bedrock layers. This material is not feasible to remove by TSHD or Backhoe Dredger (BHD) methods without first fracturing the rock. As such a drilling and blasting operation is required.

One or two non-self-propelled jack up barges or similar vessels are positioned by tug or another self-propelled vessel, and secured, usually via spud legs. The jack up will support a moveable drill tower (see Figure 14).

The drilling and blasting works have been done from a platform on the jack up with the drill tower using the following procedure.

Drilling is carried out using hydraulic drilling towers fitted with top hammer rotation units and down-the-hole hammers. The drill towers run on rails fitted to the deck of the pontoon giving them full coverage over one side of the pontoon (longitudinal)

The top hammer rotation units is used to drive the casing and rotate the drill string. The drill strings is fitted with down-the-hole hammers to maximise drilling power and the rate of penetration. The down-the-hole drill hammers is powered pneumatically.

A typical work procedure entails that the holes are drilled in a sequence and such that the holes adjacent to the longitudinal free face are drilled first, thus allowing the shot to be safely fired even if the full pattern has not been completed. Stemming is used for confining the explosive energy within the blast holes to more effectively fragment the rock. Blasting does not occur over the whole of the bay but in distinct areas where rock is present, see Figure 14.

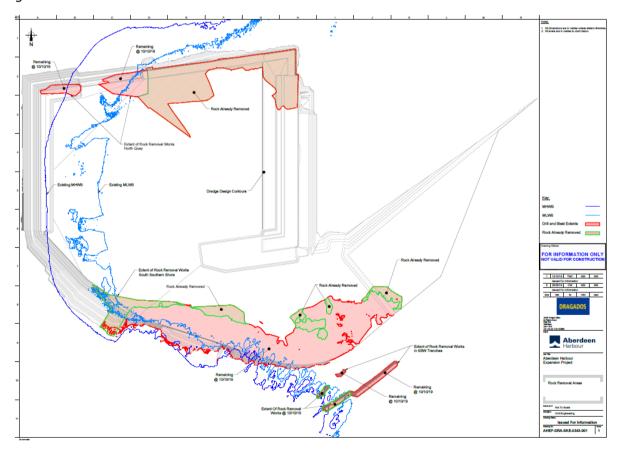


Figure 14 - Areas where rockbreaking has taken place (green) and is due to take place (red).



Once the explosives have been detonated, a BHD vessel removes and loads the blasted material in barges for reuse at AHEP (see Figure 15). It is likely that a non-self-propelled BHD is positioned by a tug or other self-propelled vessel. The BHD is fixed to a location by three spud legs. The spud legs together with an excavator bucket located on the BHD are capable of "walking" in parallel lines.

Working in lanes, the excavator positioned at the tip of the barge removes material over 5m lengths into self-propelled split barge vessel(s). The excavator is controlled via GPS systems for optimum control, with the other controls being similar to that of a land based alternative. Once the material within the reach of the excavator has been removed, the vessel removes its front spud leg from the seabed and "walk" backwards to uncover new ground.

As an alternative, rock material removal could be ripped using an hydraulic ripper in the backhole or can be demolish with a trepano in the split barge lifting with a crane installed in the vessel. Another alternative could be an hydraulic hammer used to break the rock.



Figure 15 - BHD and Split Barge

#### 5.3.5 Land Based Drill and Blast Operations

For areas around the southern slopes of Nigg Bay, South breakwater trenches, North East corner closed quay and at the north open quay, it is possible to undertake blasting from land.

Activities within areas below the current Mean High Water Spring are managed through the Marine Licence.

Activities within areas above the current Mean High Water Spring Level are out of the Marine Licence scope.

Earthwork access platforms will be constructed which will also be used as access / haul roads and lay down areas. Holes will be drilled through the platform into the rock layers below and then explosives set into these holes. Once blasting is completed the fractured rock will be excavated and used to either create the next stage of the rock platform and



ultimately in construction of the harbour such as placement in the southern breakwater or elsewhere. All blasting will be below the constructed platform and as such will not require containment.

The maximum charge weight to be used during the remaining blasting programme is limited by DUK's obligation to achieve the currently agreed threshold of 170 dB re 1  $\mu$ Pa rms (equivalent to 183 dB re 1 $\mu$ Pa peak) at 400 m from the blast location or outside the double bubble curtain, whichever is the greater distance, or some revised threshold in light of new guidance (i.e. NMFS, 2018, Southall et al, 2019)

#### 5.3.6 Dredge Deposit Volumes and Deposit Site

Dredge Area	License Depth m	Area m²	Approximate Dredge Volume m³	Approximate Dredge Volume Wet Tonnes
Harbour Basin	-9	287,348	2,254,627	4,841,515
North Quay	-9.85	12,033	39,600	85,035
East Quay	-12.95	53,205	258,700	555,525
Entrance Channel	-10.5	93,425	95,900	205,932
South East Pier	-11.45	15,945	102,500	220,105
SBW Roundhead	-15.24	9,602	12,200	26,198
West quay	-12.55	Included within harbour basin		
Total		471558	2,762,527	5,932,163*

<sup>\*</sup> Total Includes dredged material to be reused in the construction of AHEP so maximum deposit at CR110 will not exceed 4,702,737 wet tonnes / 2,190,000m<sup>3</sup>

Table 4 - Dredge and deposit volumes and deposit site

#### 5.4 QUAY INSTALLATION

The quay arrangement will be constructed in one of two ways:

- A closed quay solution spanning the south east, east and north quays, and
- An open quay solution spanning the west and western section of the north quay.

The closed quay will provide around 897m of berthing capacity with the open quay around 535m.

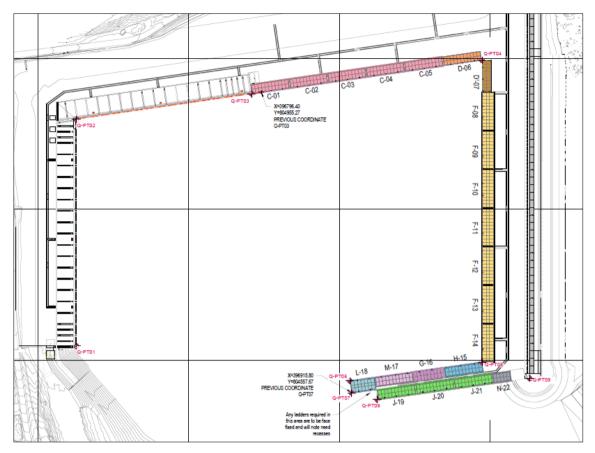


Figure 16 - Closed and Open Quay General Arrangement (North top of page)

#### 5.4.1 Closed Quay

The closed quay will be formed by 22 concrete caissons. These vary in dimensions to reflect specific loads and harbour characteristics they must represent when placed. A caisson is formed with hollow concrete cells and is typically 51.35m long. It has variable widths up to 14.7m and 16.5m in height (see Figure 17). Once positioned, the hollow cells are filled with water to "sink" the caisson. Next the caissons are filled with granular material to ground it on the seabed floor to form a permanent quayside structure.

The structure of the caissons will be formed offsite in Spain.

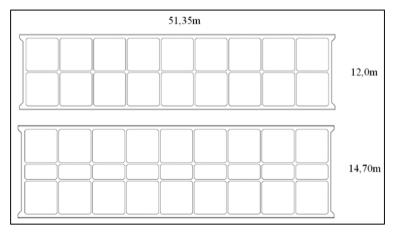


Figure 17 - General caisson arrangement



#### 5.4.2 Towage to UK

The caissons have been delivered to the UK in groups of seven to eight on a semi-submersible vessel (Figure 18). A total of 22no caissons have been delivered. The caissons were shipped directly to Scotland (observing acceptable weather conditions) where they were grouped with mooring lines for storage and later towage to site (see section 5.4.3). The total of 22no caissons were unloaded and temporary stored in Cromarty.

The risk of introduction of non-natives have been managed through implementation of risk assessments as described in the Marine Invasive Non-Native Species and Biosecurity Management Plan.



Figure 18 - Submersible vessel (indicative)

#### 5.4.3 Towage to Site

Towage to site will be completed by tug boat. The caissons will be secured and pushed/pulled to more sheltered conditions within the bay which will be protected by the north and south breakwaters. In order to install them the caissons will be towed and installed immediately in their final locations.



Figure 19 - Towage of Caissons to site (indicative)



#### 5.4.4 Caisson Embankment (Final Location Preparation)

Prior to positioning the caissons on site the dredging activities documented in section 5.3 have been done to form a trench below the caisson footprint. To provide continuous contact between the dredged trench and caisson base, a rock embankment and fine bedding layer is required (*Figure 20*).

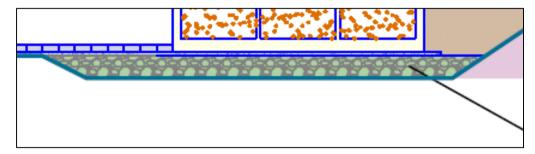


Figure 20 - Caisson and sea bed interaction

Layers of rock and bedding have been installed by a marine vessel outfitted with an excavator, crane mounted grab or conveyor system which is manoeuvrable in all directions to place the material to precise accuracy (Figure 21).

The vessel will be loaded with material before setting sail to the installation location where in depth survey and discharge operations will commence.



Figure 21 - Marine discharge vessel for rock and bedding material layers

#### 5.4.5 Sinking of Caisson in Position and scour protection placement

The sinking of caissons in their final position and mooring with anchors, will be conducted by filling all the caissons cells with sea water. Ballasting is undertaken by filling three independent groups of cells in order to maintain control the sinking operation. Once the caisson has been filled with water and sunk, two methods can be used to secure the caisson.

The first scenario uses dredged materials and rock material and various items of plant.
 A spud pontoon or other vessel loaded with a crane/excavator/conveyor/grab will
 positioning itself next to the caissons seaward boundary. A hopper barge will then
 position alongside the pontoon which will contain the dredged and rock material as



loaded by a BHD vessel. Once positions are confirmed, the cable crane fitted with a bucket or other equipment will transfer the material from barge to caisson.

 The second scenario uses sand and silt material. A TSHD will position a maximum of 200m from the caisson. The vessel will then connect via a floating pipeline to a landside network which will have an output within the caissons. A series of jets will then fluidise the material stored from dredging activities and the vessel will pump this through the pipelines into the cells. It is also possible to transfer materials by conveyor

In both scenarios the caissons are full of water before being filled with granular material. During the filling of the caissons the water is displaced by the granular material. On completion of the caisson installation, quarry supplied stone, proprietary scour mattresses and in situ underwater concrete will be placed at the outside edges on the caisson to prevent material being washed out from caisson footprint.

#### **5.4.6 Quay Transition Structures**

There are two transition structures to be formed. One will be between the north open and closed guays and the other will be between the South East Pier and the North breakwater.

The transition structure will be composed by a number of precast concrete units (2.0m  $\times$  2.0m  $\times$  2.3m) as shown in (Figure 22) and (Figure 23). These units will then be arranged in a closed box-like form and backfilled with concrete (Figure 24). This enclosed boxes will be built up in 6no levels up to +5.8m ACD approx.

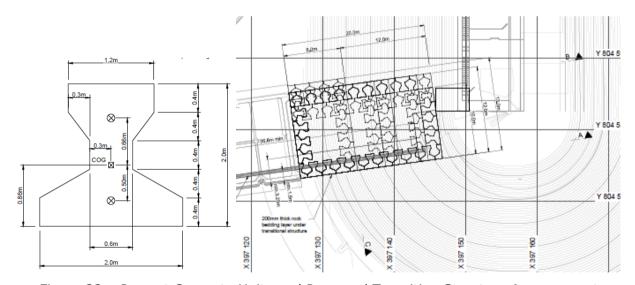


Figure 22 - Precast Concrete Units and Proposed Transition Structure Arrangement

This element of work will involve pouring underwater concrete (concrete class C25/30) as part of the backfill operations within the precast units. It is expected that the concrete pour will be filled half way on each level meaning that construction joints (as shown in Figure 23 below) with the transition structure are expected to be formed during this process.

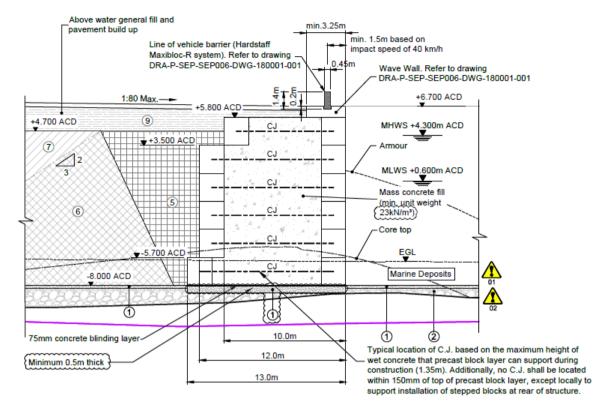


Figure 23 -Transition Structure at North breakwater showing Construction Joints



Figure 24 - Precast Concrete Units section underwater and layers above water

## 5.4.7 Open Quay

The open quay will be formed by a series of concrete piles capable of supporting the quay deck. The deck is formed of a transversal and secondary beam arrangement with a false work and concrete slab system (see 5.4.6.3). The formation of a revetment profile (see 5.4.6.2) introduces the open properties of the quayside which have wave absorption properties. A piling platform along the open quay will be built to facilitate piling. This platform will be a width of 15 metres.



Figure 25 - Open Quay Details

## 5.4.7.1 Installation of Rotary Piles

The piling platform has been created, the piles behave been installed using a rotary bored cast-in-situ concrete method (please see Piling Management Plan for detailed information). The process is as follows:

- The drill auger excavates the soil and rock to create an open bore at the required design depth
- To prevent collapse of the bore, temporary steel casing will be installed into the ground.
   In the case of deep bores where temporary steel casing may not be suitable, the use of a support fluid such as vinyl polymer or, more commonly, bentonite drilling fluid may be used
- Once the auger has reached design depth a cleaning bucket is used to ensure cleanliness of the base
- If would be necessary bentonite be used to support the bore then the slurry is recirculated and replaced within the bore to avoid any impact on concrete quality
- The reinforcement cage is lowered into the open bore
- Concrete is delivered into the bore by discharge into a hopper feeding a tremmie pipe. The concrete is poured from the base of the bore to surface
- The temporary steel casing is removed, leaving the concrete pile in situ

#### 5.4.7.2 Formation of Revetment

On completion of pile installation in local areas, the revetment profile will be formed using a long reach excavator to form the required slopes. Where the excavator cannot create the slope, dredging equipment will be mobilised. The revetment also extends round the south west corner of the southern shore

The excavated surface will then be protected with an initial filter layer, installed using a land and marine based excavator and/or a rock skip/tray suspended from a crane. The primary



layer is placed over the initial filter using an excavator and/or a grab suspended from a crane capable of placing 1000-3000kg rock (Figure 26).



Figure 26 - Methods of rock placement

### 5.4.7.3 Placement of Transversal and Secondary Beams

Transversal and secondary beams will be placed by a mobile crawler crane. The transversal beam will be placed firstly after a hydraulic cutter has been used to crop the pile heads. The beam will then be positioned over the two pile heads with any gaps sealed with concrete once the diaphragm is poured. Once two transversal beams have been installed the secondary beams can be installed one at a time using an additional crawler crane. The beams will be installed individually working from the landside out towards the quayside. The same crane will then be utilised to install the permanent formwork between the secondary beams to support a slab pour (Figure 27).

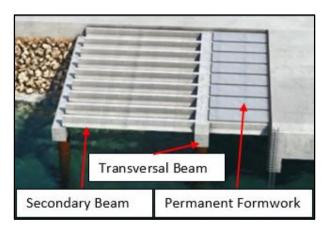


Figure 27 - Open Quay Beam Arrangement

### 5.4.8 Placement of Deck Slab

The deck slab will be poured on top of the beam and formwork arrangement shown in Figure 20. A system of temporary formwork will be constructed in panels to contain a pumped concrete mixture. On successful curing, the formwork will be removed and repositioned for the next pour.

#### 5.4.9 Reclamation

Reclamation activities will be carried out by one of two methods, either importing quarry material or using suitable site won material.



## 5.4.10Reclamation Using Imported Material

Dump trucks and quarry vehicles will progress to discharge material at varying locations dependant on the reclamation progression. The material will be pushed by a bulldozer to the required location where an excavator will grade the material to form the required profiles for installation of further material.

#### 5.4.11 Reclamation Using Locally Dredged Material

Where the preference is to use locally dredged material, a dredger will position a maximum of 200m from the reclamation. The vessel will then connect via a floating pipeline to a landside network which will have an output within the area required. A series of jets will then fluidise the material stored from dredging activities and the vessel will pump this through the pipeline. After each discharge, the vessel will disconnect, continue to dredge and then reconnect. This material will be consolidated by compaction techniques..



Figure 28 - Pumping ashore via pipeline system (Van Oord Costa Dorada)

#### 5.4.12 Pavement

The quay pavement will begin on completion of the reclamation, the pavement will be completed by direct discharge of concrete trucks in a controlled manner which will be finished and controlled by specialised items of plant (Figure 29). The plant will be guided, with expansion and crack joints introduced by road saws at later dates.



Figure 29 - Pavement construction activities

#### 5.4.13 Surface features

Harbour infrastructure above +4.7m is referred to as part of the 'Surface Features'. Key elements associated with this are;

- Service Trenches
- Harbour Drainage
- Water Supply Infrastructure
- Electrical Distribution
- Security Infrastructure
- Weighbridges
- Harbour Buildings

In specific areas, notably drainage outfalls and service trench will extend below +4.7m as noted below.

#### **5.4.14 Service Trenches**

Service trenches are provided to distribute piped services to moored vessels via bunkering pits around the quay as required by AHB.

Service trenches accommodate water supply pipework, fuel supply pipework and in some cases fuel/oil discharge pipework. Space is provided for specified future pipework for uses such as transfer of drilling mud.

Service trenches are proposed to be constructed in concrete with heavy duty removable covers at key locations to enable installation and ongoing maintenance and repair.

#### 5.4.15 Harbour Drainage

The harbour storm drainage network is a gravity drainage system with the quay areas drained to continuous industrial-grade slot drains with collection pipework running through oil interceptors prior to discharge to sea.



In locations where it is necessary for the drainage to cross the service trench, the drainage will pass beneath the services trench, dictating the level mainly for the outfalls.

# 5.4.16Water Supply Infrastructure

Within the harbour area provision is made for water storage for supply to visiting vessels. Supply pipework is to be provided via the service trenches noted above.

## 5.4.17 Electrical Supply Distribution

Distribution infrastructure will be incorporated in the works to facilitate the distribution of electrical power around AHEP. This duct network will be integrated with that required to supply harbour lighting and comms systems (subject to the required separation and segregation).

## 5.4.18 Security Infrastructure

Key elements of security infrastructure to be installed include;

- · Main Gate Entry Barriers and controls
- Harbour perimeter security fencing
- CCTV provision and monitoring

## 5.4.19Weighbridges

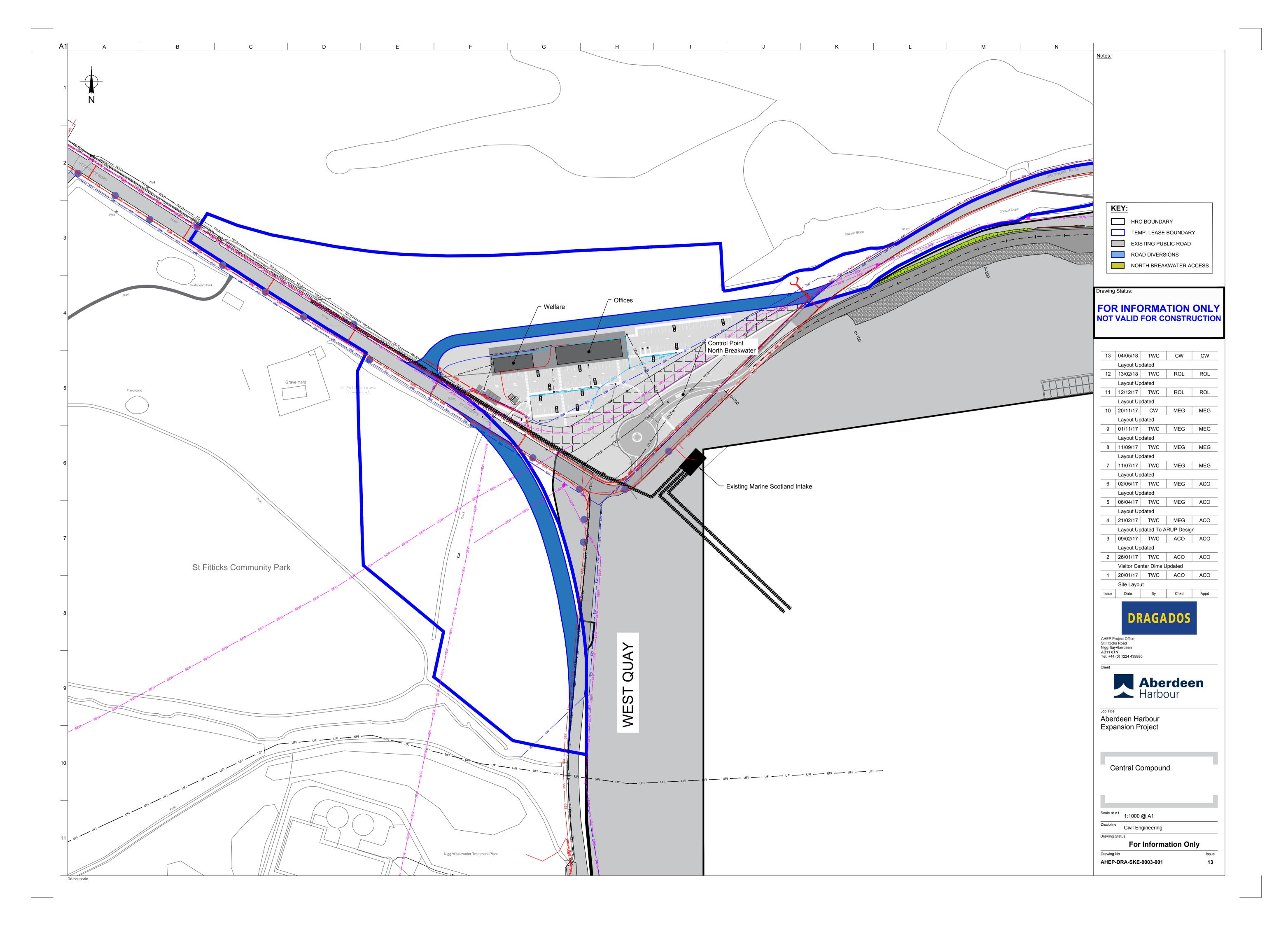
Provision of 2 weighbridges will be made in accordance with AHB requirements.

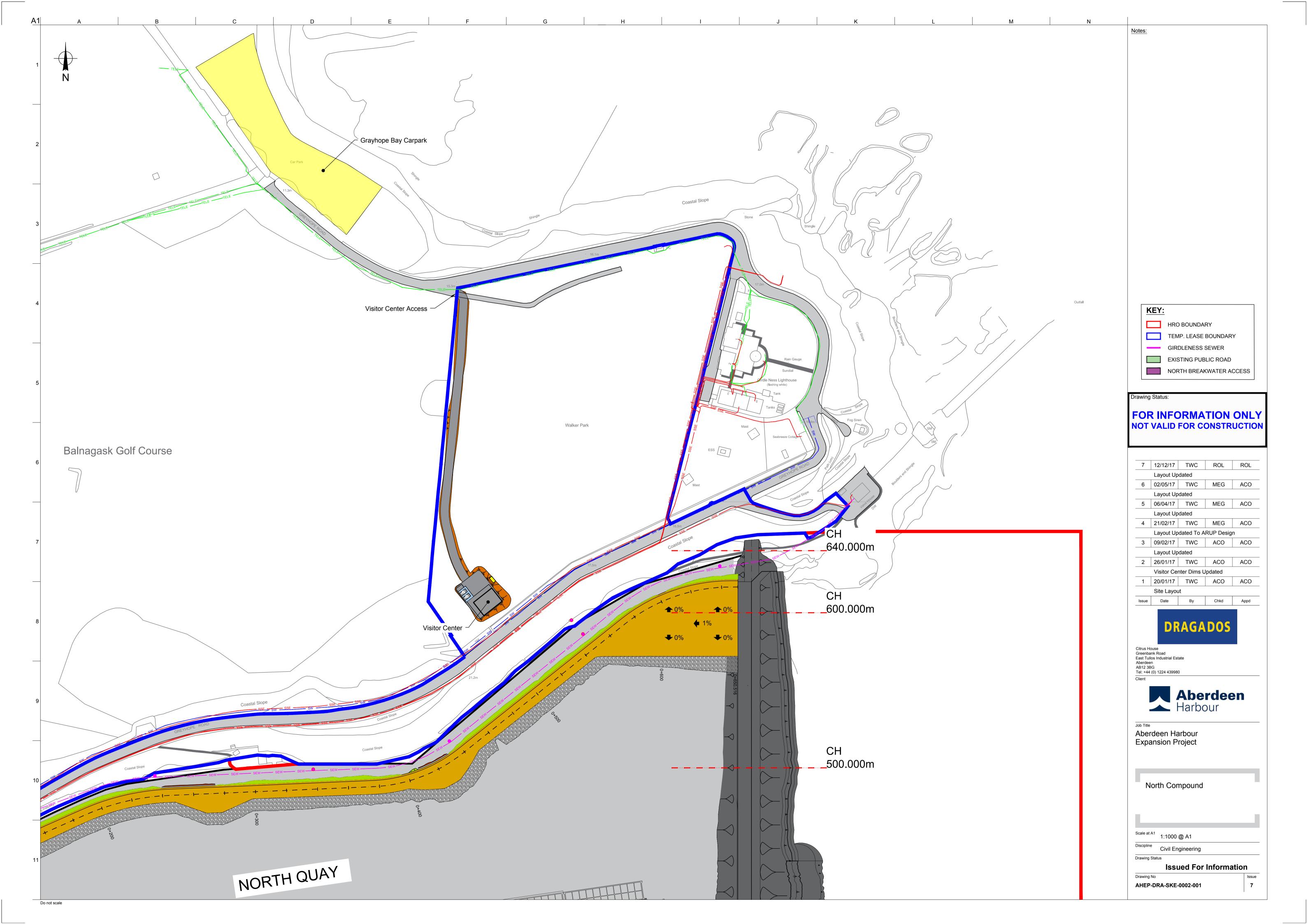
## 5.4.20 Harbour Buildings

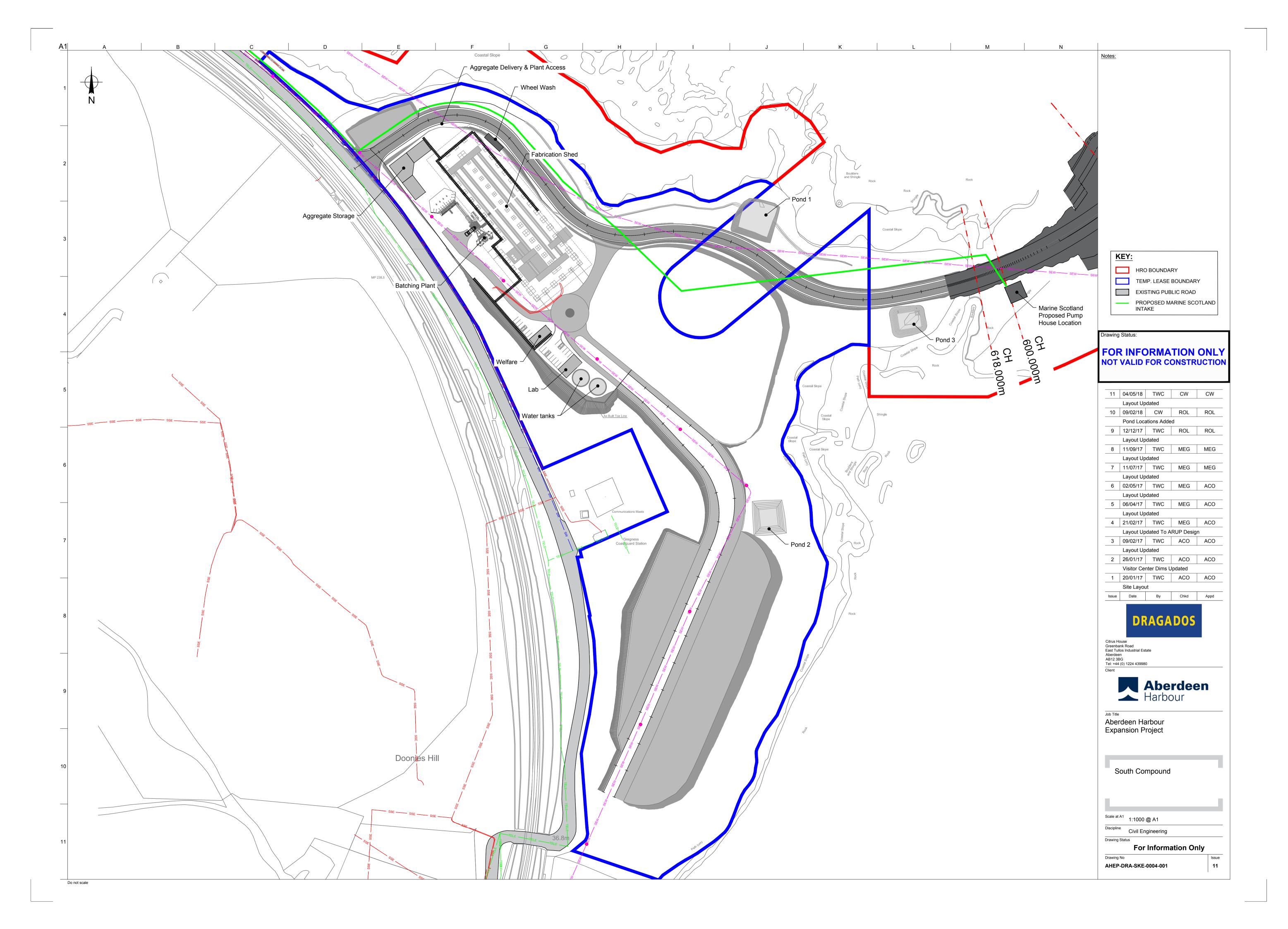
Provision of buildings on the harbour will be made in accordance with AHB requirements, to include entrance security provision and on-site welfare accommodation.



# **APPENDIX 1 A, B AND C - COMPOUNDS AREAS LAYOUT**

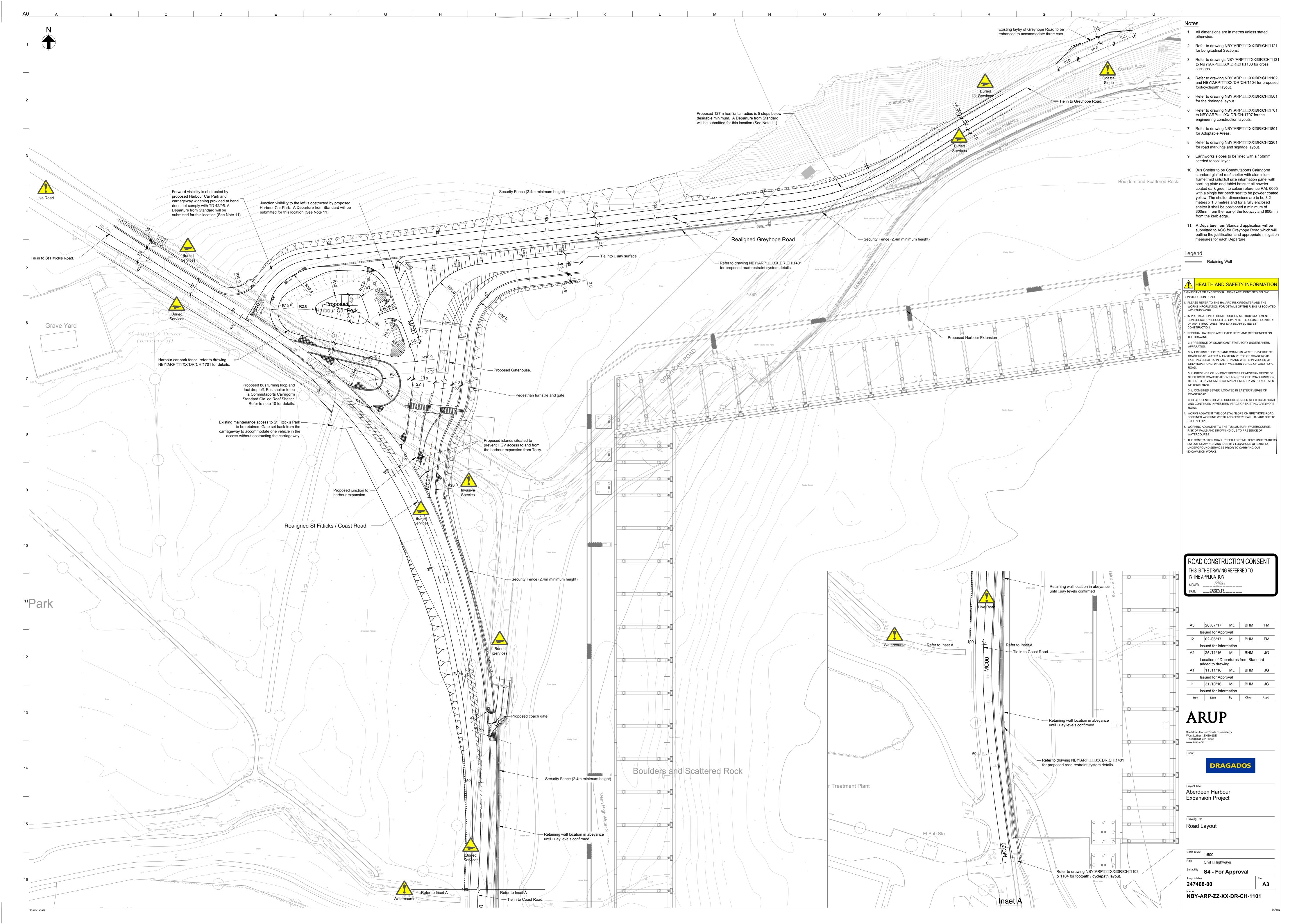


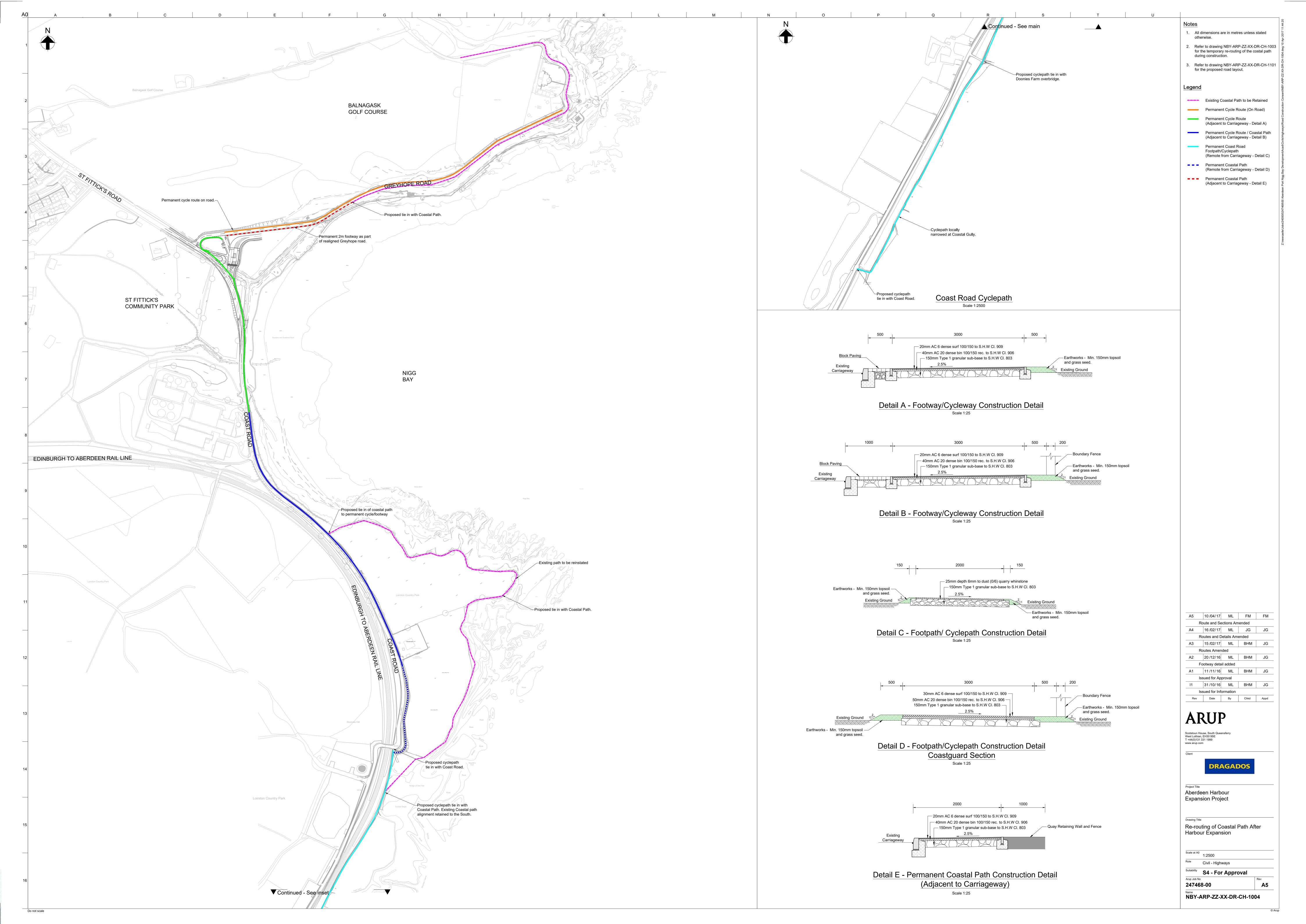






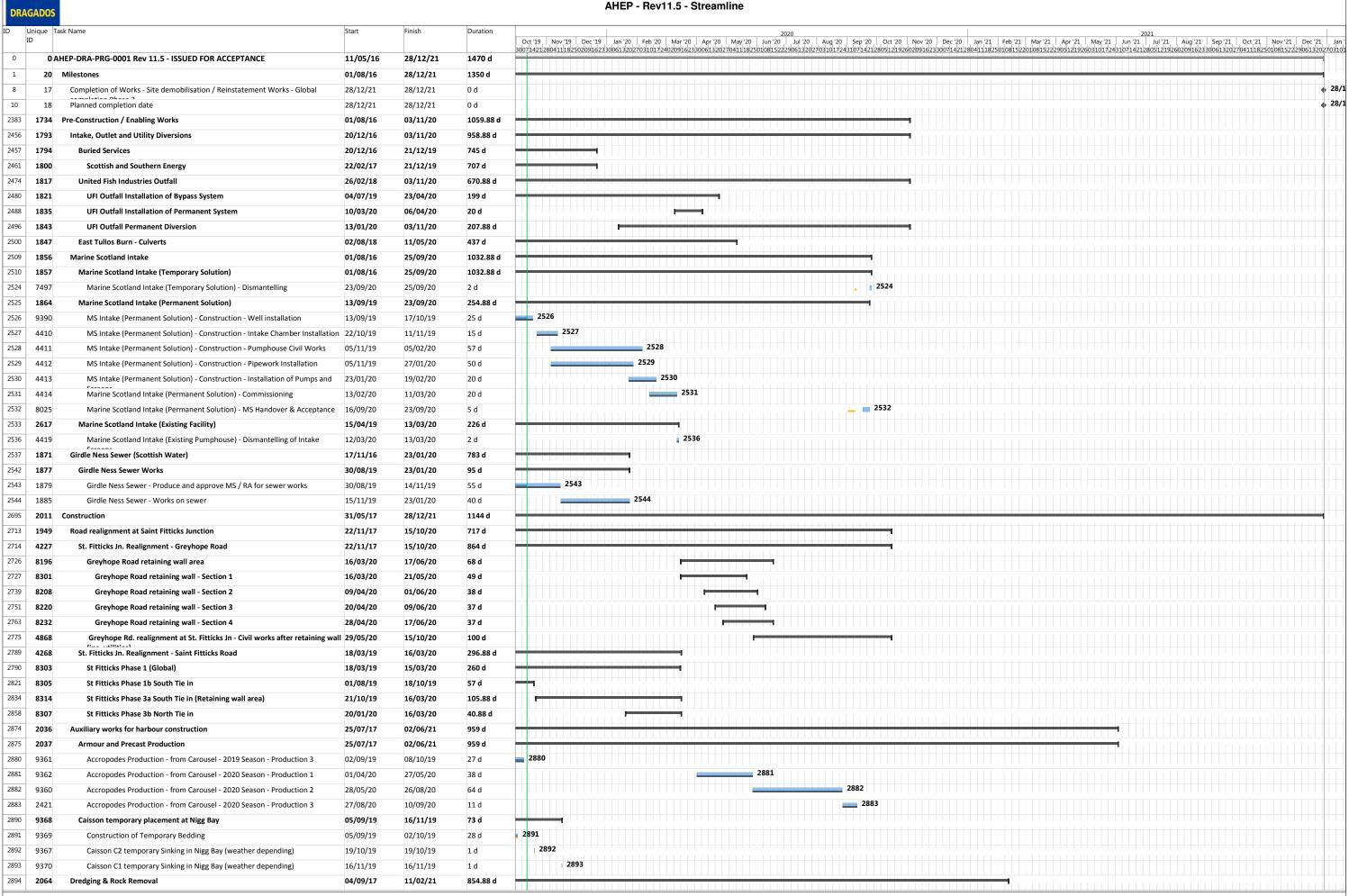
# **APPENDIX 2 - ROAD WORKS LAYOUT**

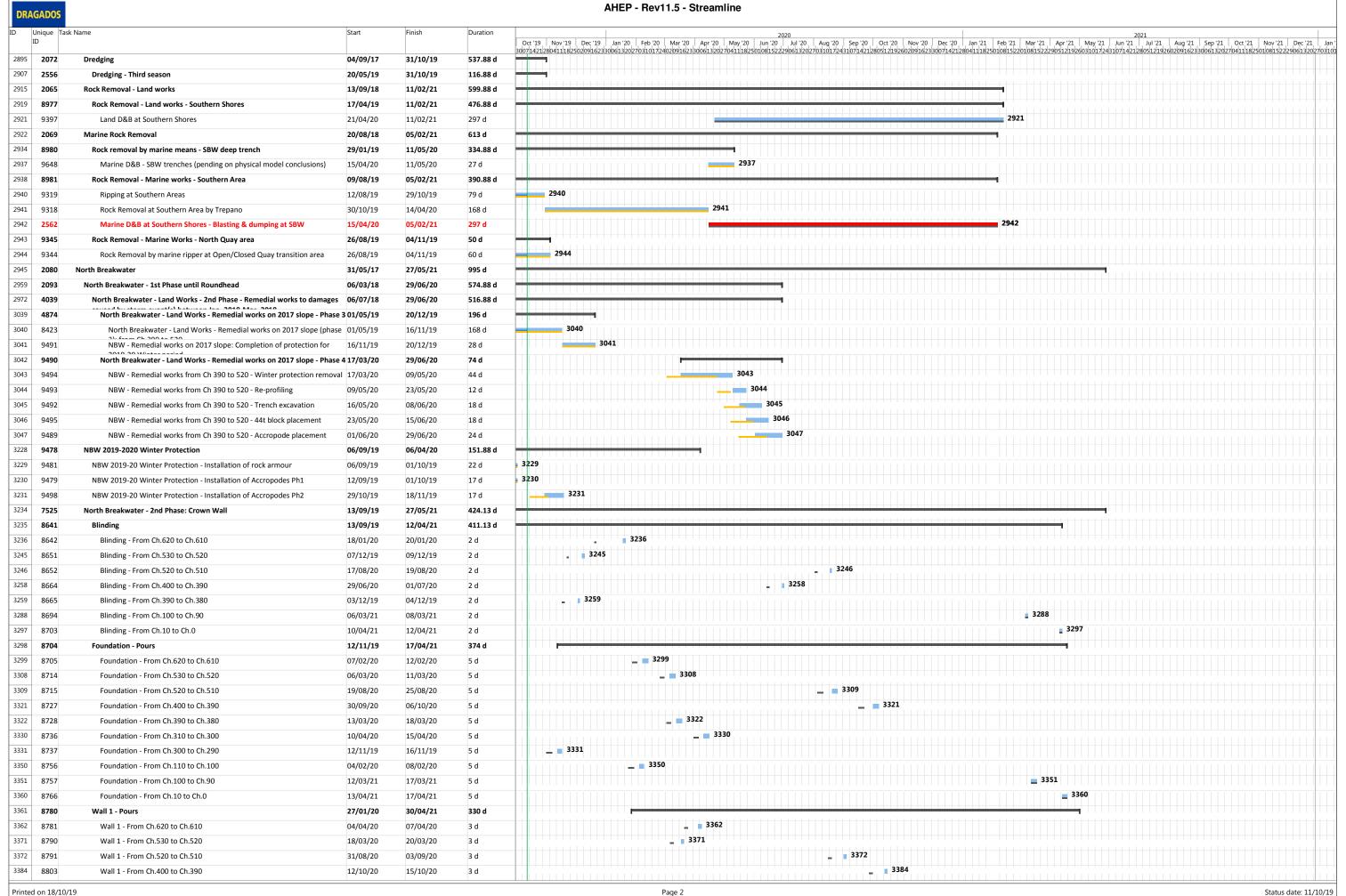


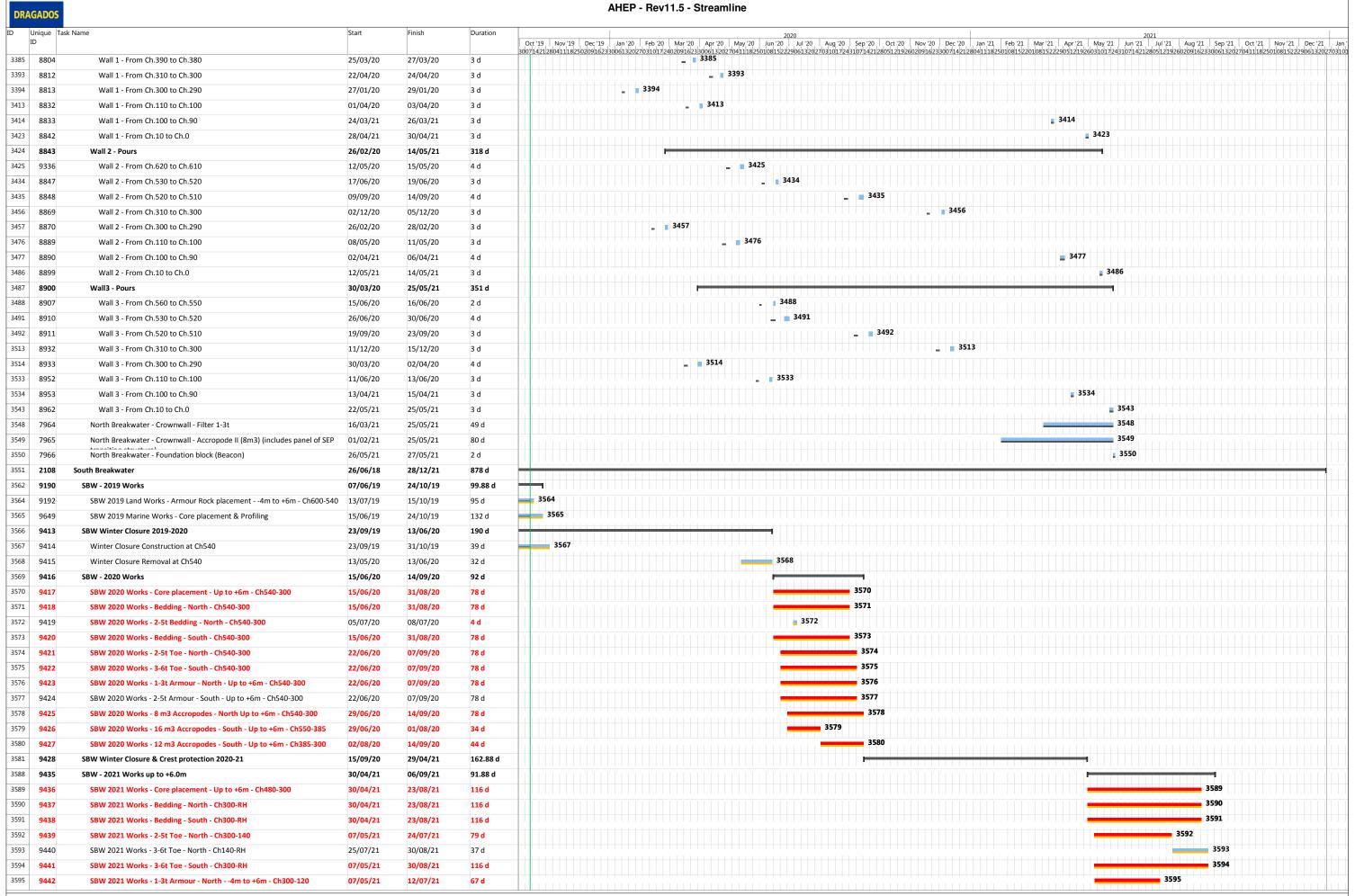


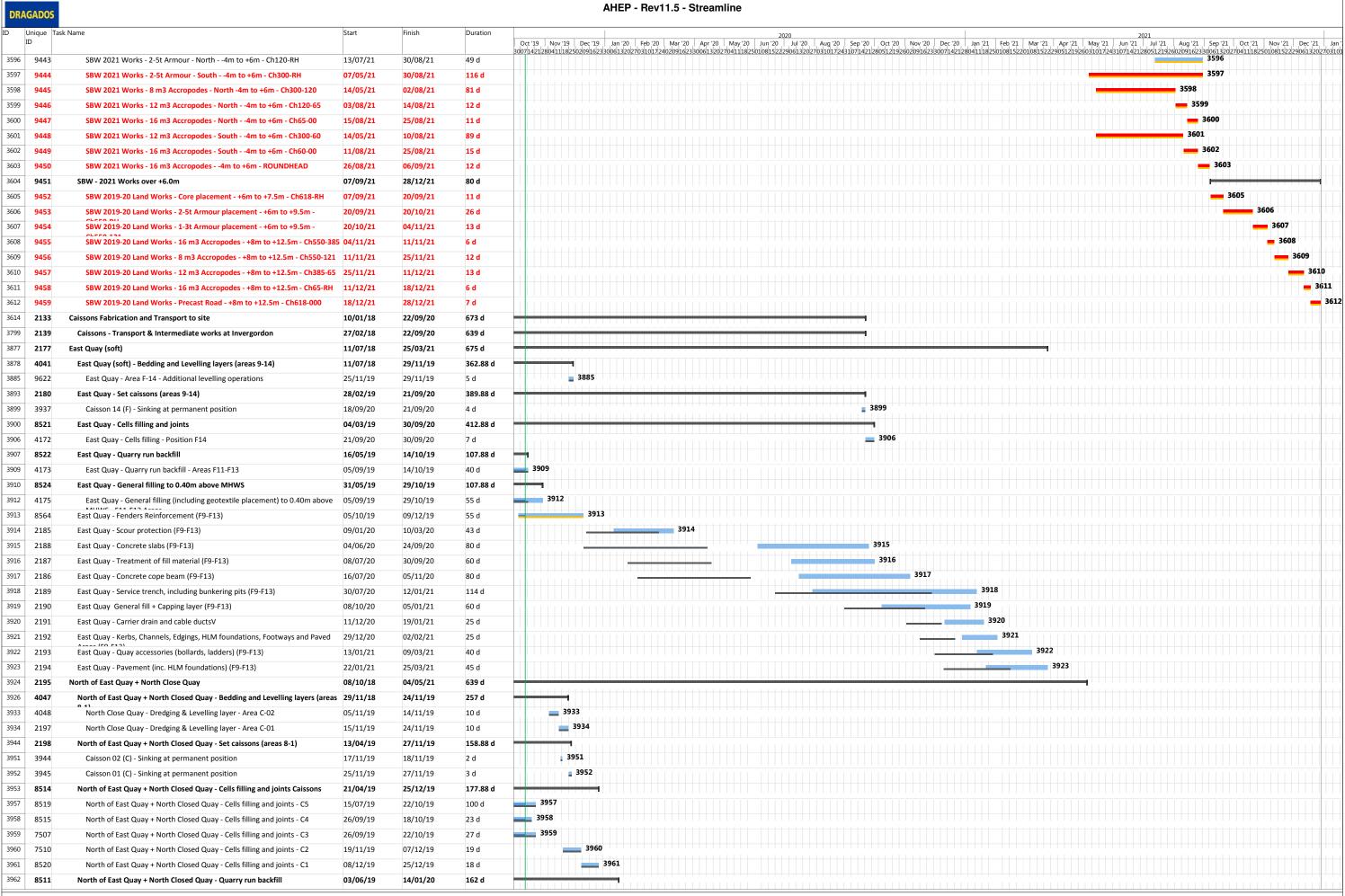


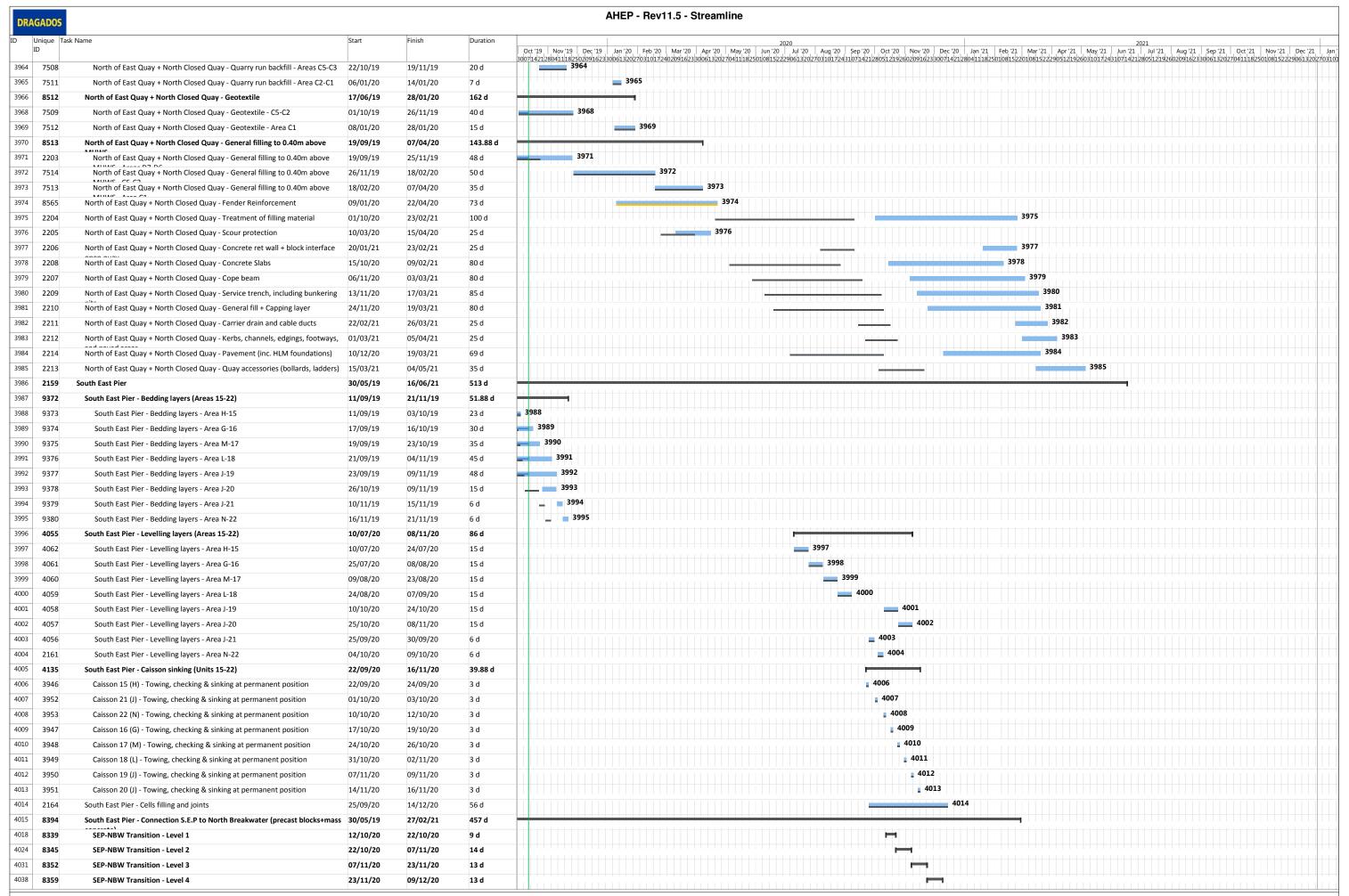
# **APPENDIX 3 - PROGRAMME**





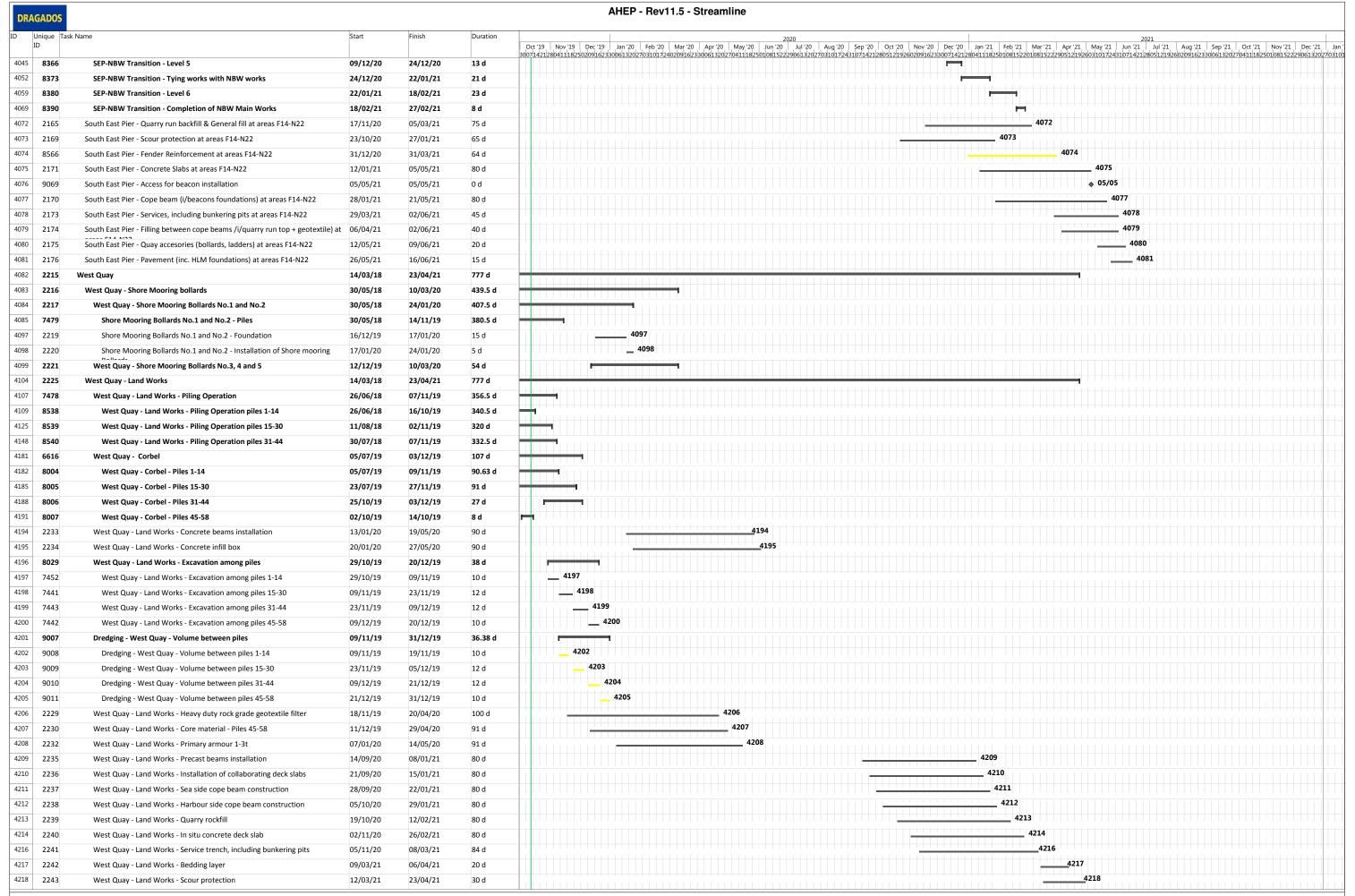


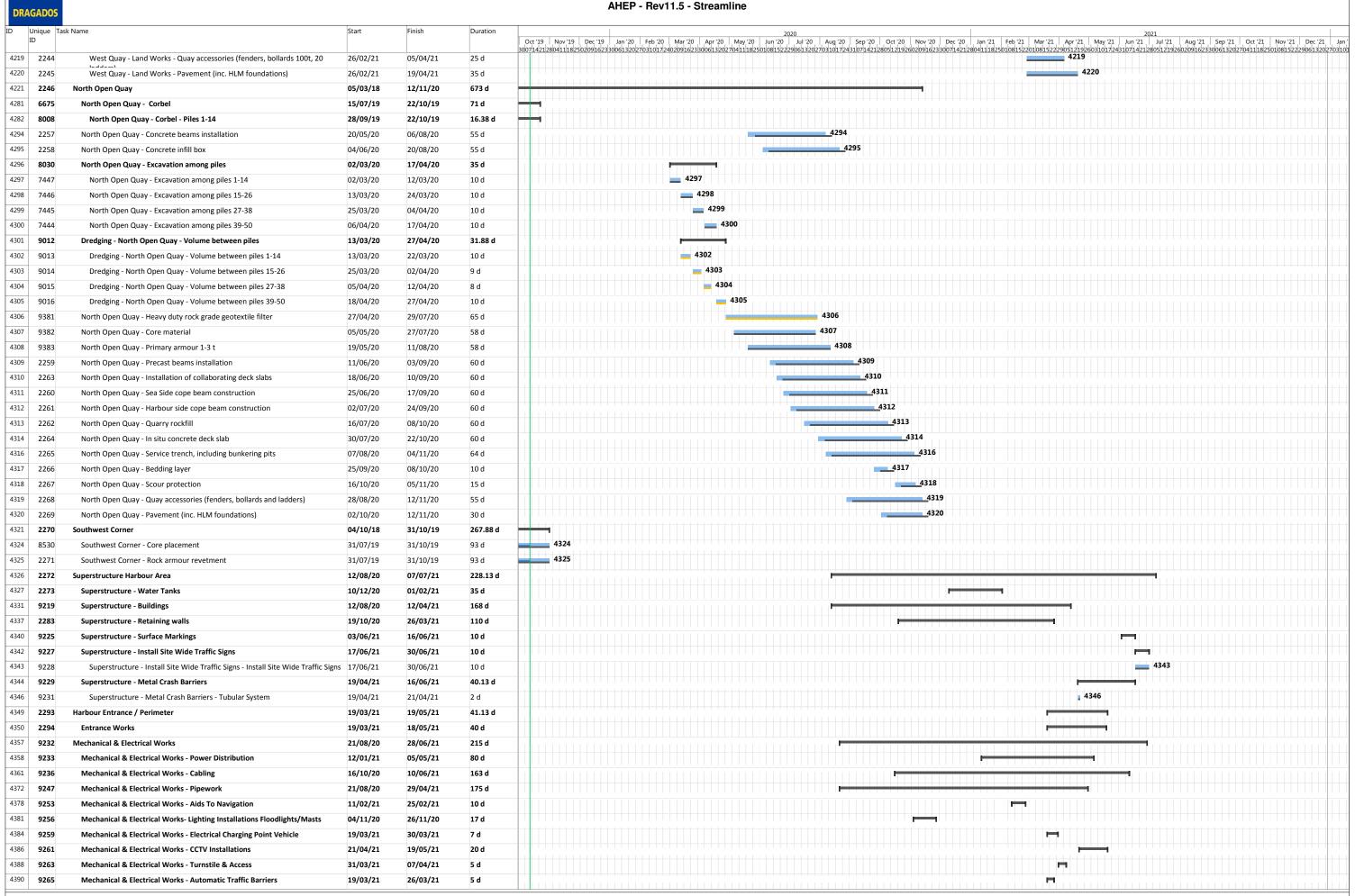




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4398	9273	Mechanical & Electrical Works - Commissioning	12/03/21	28/06/21	73.13 d														-			-			
4404	9279	Mechanical & Electrical Works (by AHB)	01/11/21	27/12/21	40 d																			-	
4405	9280	Mechanical & Electrical Works (by AHB) - Aids to Navigation	01/11/21	24/12/21	38 d																				
4409	9284		01/11/21	12/11/21	10 d																			4	409
4412	9287	Mechanical & Electrical Works (by AHB) - Turnstile & Access	01/11/21	27/12/21	40 d																				
4414	9289	Mechanical & Electrical Works (by AHB) - Automatic Traffic Barriers	01/11/21	27/12/21	40 d																			-	
4416	9291	Mechanical & Electrical Works (by AHB) - Weighbridge Installations	01/11/21	27/12/21	40 d																			-	<del></del> -
4418	9293		01/11/21	27/12/21	40 d																				
4420	9295	Mechanical & Electrical Works (by AHB) - Communications	01/11/21	27/12/21	40 d																			-	
4422	2331	Finishing Works	29/06/21	28/12/21	128 d																	-			
4423	2332		29/06/21	28/12/21	128 d																				4423
4424	2351	Demobilisation & Reinstatement of Compounds	15/11/21	28/12/21	30 d																			F	
4425	2352	Compounds demobilisation	15/11/21	28/12/21	30 d																			-	
4426	2353	Demobilisation - South Compound Area	15/11/21	28/12/21	30 d																			_	4426
4427	2354	Demobilisation - Central Compound Area	29/11/21	28/12/21	20 d																				4427
4428	2355	Demobilisation - North Compound Area	15/11/21	14/12/21	20 d																				4428
4429	2356	Permanent Coastal Path rerouting and upgrading	10/12/21	28/12/21	12 d																				
4430	2357	Potential permanent Path Around Walker Park	10/12/21	24/12/21	10 d																				4430
4431	2358	Permanent Path Around Loirston Park	14/12/21	28/12/21	10 d																				4431